



Traffic Impact Analysis

3rd Submittal

Apache Trails
Multi-Family Residential

Plaza Drive & Apache Trail

Apache Junction, Arizona

May 2025

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Prepared by:

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EXECUTIVE SUMMARY

INTRODUCTION

The multi-family residential development is proposed east of Plaza Drive between Superstition Boulevard and Apache Trail in Apache Junction, Arizona. The development is proposed with three-story buildings primarily consisting of one and two-bedroom units with a total of 336 dwelling units. The primary access is proposed on Apache Trail. A secondary access is proposed on Plaza Drive north of the existing RV Resort driveway and will be limited to a southbound left-in and westbound right-out only. Both entrances will be gate controlled.

The traffic impact analysis was prepared in accordance with the requirements of the City of Apache Junction. The proposed development generates more than 100 peak hour trips. The analysis study years include existing conditions and horizon years 2026 and 2031. An additional annual 5% growth rate was used to estimate the growth of ambient background traffic in the horizon years (results in a 41% total increase by 2031).

This report has been updated to address the seasonality of the collected traffic counts and to discuss the difference in peak hour traffic generated from multi-family residential developments versus the existing peak hour at the study intersections.

SEASONAL FACTOR

The City of Apache Junction annually collects traffic volumes in February or March and is available to the public. Additionally, both the Arizona Department of Transportation (ADOT) and Maricopa Association of Governments (MAG) regularly collect and compile traffic data on many roadways within their boundaries. At the applicant's request, this report has been revised to compare all recent historical traffic volumes near the proposed site from these agencies and compare them to the counts collected in September 2024 as part of this study.

There were several available counts recorded near the proposed site during both the higher traffic winter months and lower traffic summer months. Based on the evaluation of the relevant count data, an approximate 32% difference was observed between the peak and non-peak seasons. To provide a conservative evaluation, a 35% growth factor was utilized. Therefore, in addition to the typical annual background growth rate, a seasonal factor of 1.35 was applied to the existing traffic volumes collected in September for use in this analysis.

TRIP GENERATION

Full build-out of the proposed development is anticipated to generate 2,265 weekday daily trips (entering/exiting) with 134 trips (entering/exiting) during the AM peak hour and 171 trips (entering/exiting) during the PM peak hour. Existing traffic counts reveal that the existing peak hour of traffic at the study intersections generally occurs at 12:00 PM, which differs from the morning and evening peak hour periods of multi-family residential land uses.

When compared to a shopping center use allowed under B-3 zoning, the proposed multi-family development generates 5,211 fewer daily trips, 36 fewer AM peak hour trips, and 516 fewer PM peak hour trips.

LEVEL-OF-SERVICE

The study intersections operate at acceptable levels of service with existing conditions. With trips from the proposed development, the study intersections will continue to operate at acceptable levels of service in both horizon years 2026 and 2031 with the existing lane configurations and traffic control.

The two closest intersections to the proposed development's driveways are Plaza Drive / Superstition Drive and Apache Trail / Idaho Road. With the seasonal factor of 1.35 applied, the added site traffic is projected to add 3.0 seconds of delay to the northbound approach of the Plaza Drive / Superstition Boulevard intersection during the AM and PM multi-family peak hours. At the Apache Trail / Idaho Road intersection, with the seasonal factor of 1.35 applied, site traffic is expected to add 0.2 and 0.5 seconds of delay to the overall intersection during the AM and PM multi-family peak hours, respectively.

SITE ACCESS

A full access driveway is proposed on Apache Trail. A secondary driveway is proposed on Plaza Drive north of the existing RV Resort driveway and will be limited to a southbound left-in and westbound right-out only. The driveways will be controlled by a stop sign. Both Plaza Drive and Apache Trail are low volume roadways with a projected daily traffic volume less than 3,500 vpd on Plaza Drive and less than 5,400 vpd on Apache Trail. The posted speed limit on Apache Trail is 15 mph. A speed limit sign of 15 mph is provided on Plaza Drive for the existing Lost Dutchman RV Resort. As part of the proposed development, half-street improvements will be provided on Apache Trail adjacent to the property. Half-street improvements will also be provided on Plaza Drive, which will include the provision of angled on-street parking in the northbound direction.

INTRODUCTION

A traffic impact analysis is required during the development review process to assess the impacts of the proposed project on the surrounding transportation system. EPS Group has been retained to prepare the traffic impact analysis for the proposed multifamily project.

The development is proposed east of Plaza Drive between Superstition Boulevard and Apache Trail in Apache Junction, Arizona. The development is proposed with three-story buildings primarily consisting of one and two-bedroom units with a total of 336 dwelling units. The primary access is proposed on Apache Trail. A secondary access is proposed on Plaza Drive north of the existing RV Resort driveway. Both entrances will be gate controlled. **Figure 1** provides a location map, and an aerial map is provided in **Figure 2**.

SCOPE OF STUDY

The traffic impact analysis was prepared in accordance with the requirements of the City of Apache Junction. The proposed development generates more than 100 peak hour trips. The analysis study years include existing conditions and horizon years 2026 and 2031. The objectives of the study are the following:

- ❖ Document and evaluate existing traffic conditions
- ❖ Estimate and evaluate future non-site 2026 and 2031 traffic conditions
- ❖ Estimate new traffic generated by the proposed development and assign new trips to street system
- ❖ Evaluate operation of adjacent streets and intersections with the proposed development
- ❖ Determine the need for modified traffic control and auxiliary lanes at the study intersections
- ❖ Review driveway design, spacing, and on-site circulation

STUDY INTERSECTIONS

The following intersections are included in the study area:

- ❖ Plaza Drive & Superstition Boulevard
- ❖ Plaza Drive & Existing Driveway
- ❖ Plaza Drive & Site Access A
- ❖ Idaho Road & Apache Trail
- ❖ Apache Trail/Phelps Drive & Old West Highway
- ❖ Apache Trail & Site Access B

EXISTING CONDITIONS

SURROUNDING LAND USE

The proposed site is currently vacant. The existing Lost Dutchman RV Resort is on the west side of Plaza Drive opposite the site and currently exclusively uses Plaza Drive for resident and guest entry/exit and does not use their driveway at Superstition Boulevard. Retail uses and a public park border the site to the south. Horizon Health and Wellness borders the site to the north along with a United States Postal Service (USPS) office.

DESCRIPTION OF EXISTING TRANSPORTATION SYSTEM

The existing roadway geometry and intersection control is depicted in **Figure 3**. Access to the proposed development is provided on existing Plaza Drive and Apache Trail.

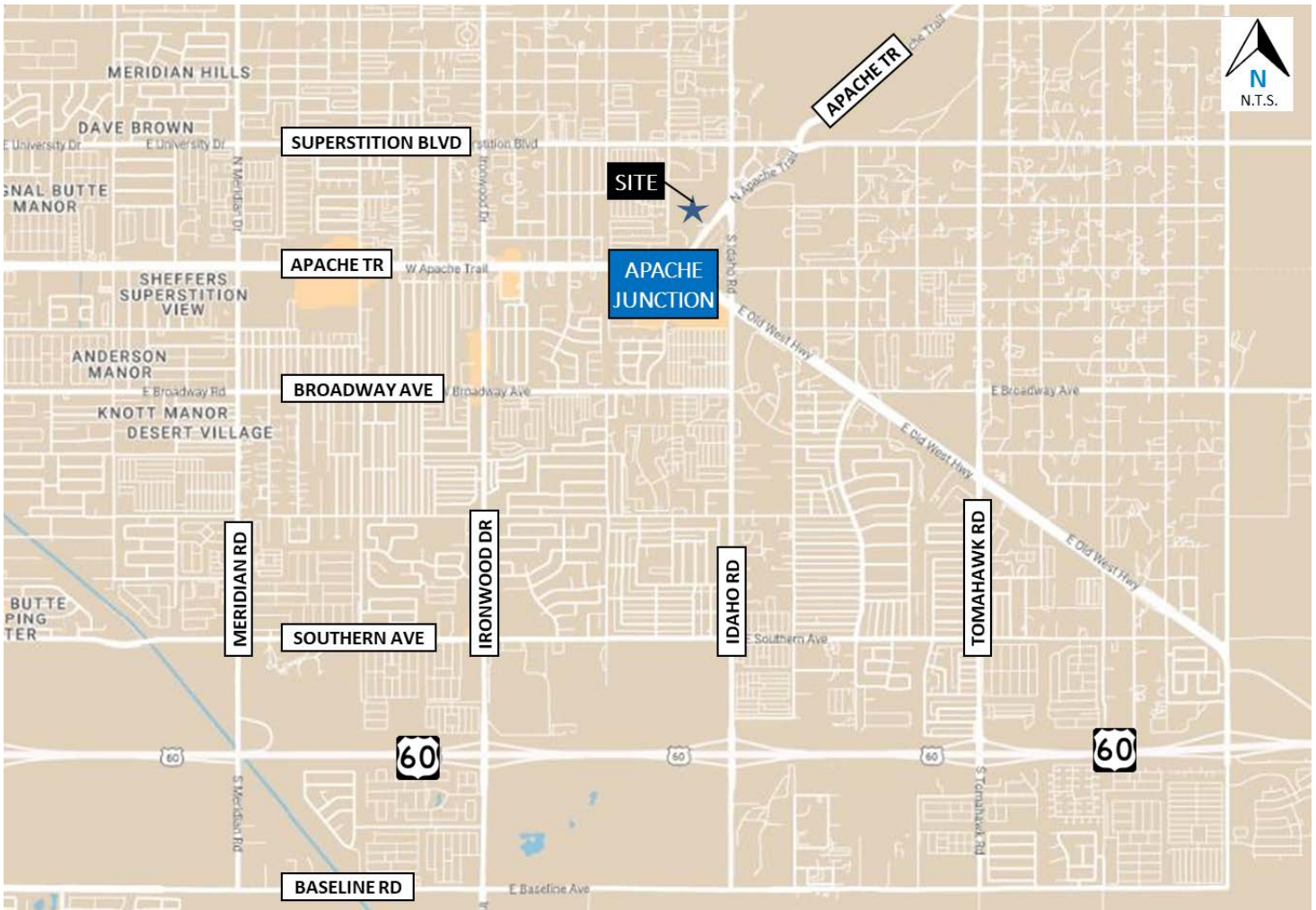


Figure 1: Location Map

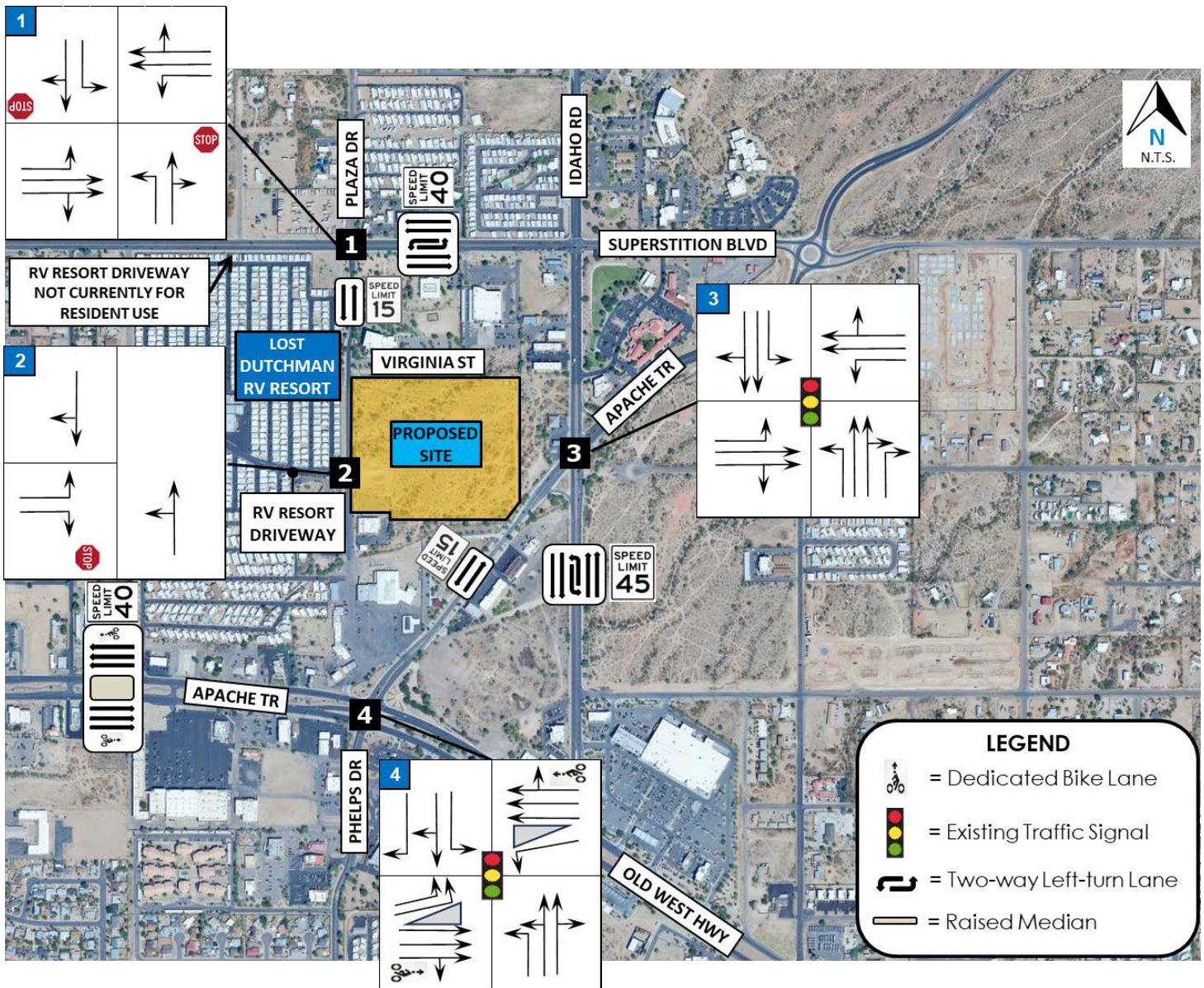


Figure 2: Aerial Map

STUDY INTERSECTIONS

The existing intersection lane configuration and traffic control is depicted in **Figure 3**, and the traffic control is also summarized in **Table 1**.

Table 1: Existing Intersection Traffic Control

Intersection	Traffic Control
1. Plaza Drive & Superstition Boulevard	Minor Street Stop Control (NB/SB)
2. Plaza Drive & Existing RV Resort Driveway	Minor Street Stop Control (EB)
3. Idaho Road & Apache Trail	Traffic Signal
4. Apache Trail/Phelps Drive & Old West Highway	Traffic Signal

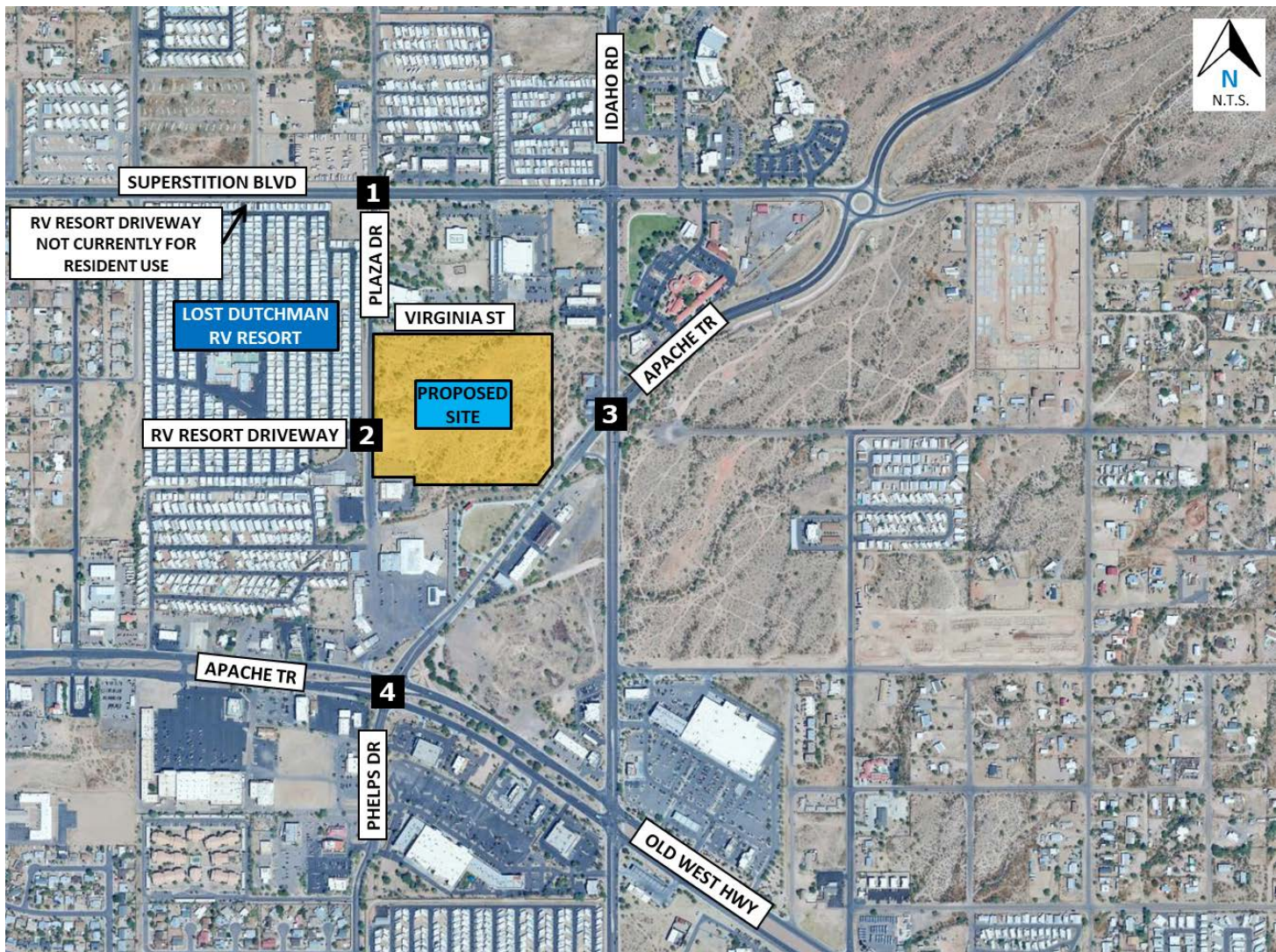


Figure 3: Existing Lane Configuration and Traffic Control

ROADWAYS

Plaza Drive

Plaza Drive is a local roadway with one lane in each direction providing access to the Lost Dutchman RV Resort and a commercial area south of the site. The posted speed limit is 15 mph near the site.

Plaza Drive right-of-way (ROW) terminates and transitions into the East Valley Shopping Center plaza private property. Traffic volumes were not collected in this area since it is not within the ROW, and therefore not intended for local traffic use through the shopping center parking area for access to Apache Trail. However, in discussions with neighboring owners and the City, the parking area does experience through traffic other than retail shoppers, including the Lost Dutchman RV park and Skyhaven Estates RV park. The Skyhaven Estates RV park has a secondary entrance into the shopping center property. The applicant has designed the Plaza Drive entrance to avoid movements that would encourage southbound traffic on Plaza Drive.

Apache Trail

Apache Trail runs east/west before heading northeast at Phelps Drive and towards the site continuing onto the lakes and recreation areas. Near the site, Apache Trail is two lanes with one in each direction and flares out at the signalized intersections at Idaho Road and Old West Highway. Angled parking is provided on Apache Trail Adjacent to the public park south of the site. The posted speed limit is 15 mph adjacent to the site.

EXISTING TRAFFIC VOLUMES

Segment counts in each direction were collected on Thursday September 12, 2024 for 24 hours in 15-minute intervals on at the following locations:

- ❖ Plaza Drive south of Superstition Boulevard
- ❖ Idaho Road south of Superstition Boulevard
- ❖ Apache Trail southwest of Idaho Road

Turning movement traffic counts were also obtained on Thursday September 12, 2024 from 7:00 AM to 9:00 AM and from 4:00 PM to 6:00 PM at the following study intersections:

1. Plaza Drive & Superstition Boulevard
2. Plaza Drive & Existing Driveway
3. Idaho Road & Apache Trail
4. Apache Trail/Phelps Drive & Old West Highway

SEASONAL FACTOR

The City of Apache Junction annually collects traffic volumes in February or March and is available to the public. Additionally, both the Arizona Department of Transportation (ADOT) and Maricopa Association of Governments (MAG) regularly collect and compile traffic data on many roadways within their boundaries. At the applicant's request, this report has been revised to compare all recent historical traffic volumes near the proposed site from these agencies and compare them to the counts collected in September 2024 as part of this study.

There were several available counts recorded near the proposed site during both the higher traffic winter months and lower traffic summer months. Based on the evaluation of the relevant count data, an approximate 32% difference was observed between the peak and non-peak seasons. To provide a conservative evaluation, a 35% growth factor was utilized. Therefore, in addition to the typical annual background growth rate, a seasonal factor of 1.35 was applied to the existing traffic volumes collected in September for use in this analysis.

Figure 4 depicts the existing weekday traffic counts collected in September 2024, and **Figure 5** depicts the adjusted existing traffic counts with a 1.35 seasonal variation factor applied. The existing traffic counts are provided in **Appendix B** to this report. The seasonal factor calculation is included in **Appendix C**.

PEAK HOUR AND ANALYSIS TIME PERIODS

Weekday turning movement counts were collected from 7:00 AM to 9:00 AM and from 4:00 PM to 6:00 PM to correspond with the typical peak hours of a proposed multi-family residential development. The Institute of Transportation Engineers' (ITE) *Trip Generation Manual* provides data for multi-family uses (Land Use Code 220) for weekday peak hour periods occurring between 7:00 and 9:00 AM and 4:00 and 6:00 PM as these are the typical peak periods for the land use.

Although the morning and evening peak hour periods are studied in the analysis, the 24-hour roadway segment traffic volumes collected for this analysis reveal that the study intersections experience the most traffic volumes during the late morning and early afternoon with the peak hour generally occurring at 12:00 PM. The City's annual traffic volumes also reveal the midday peak hour, which may largely be attributed to the significant number of 55+ age restricted RV parks in the area not following the typical commuting patterns of non-age restricted multi-family residential uses. Thus, the peak traffic of the proposed development will not correspond with the existing peak hour traffic of the study area.

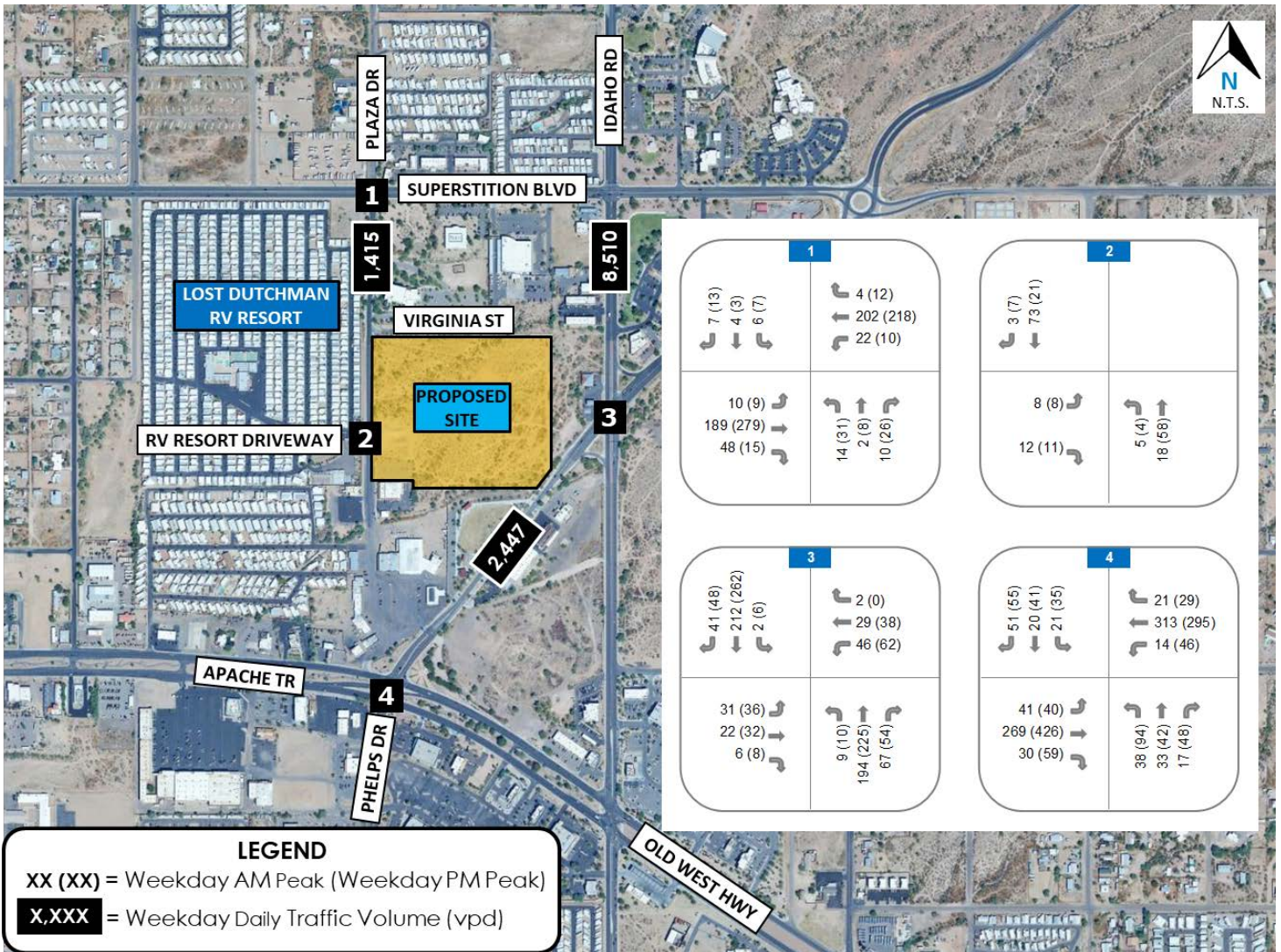


Figure 4: Existing Weekday Traffic Volumes (September 2024)

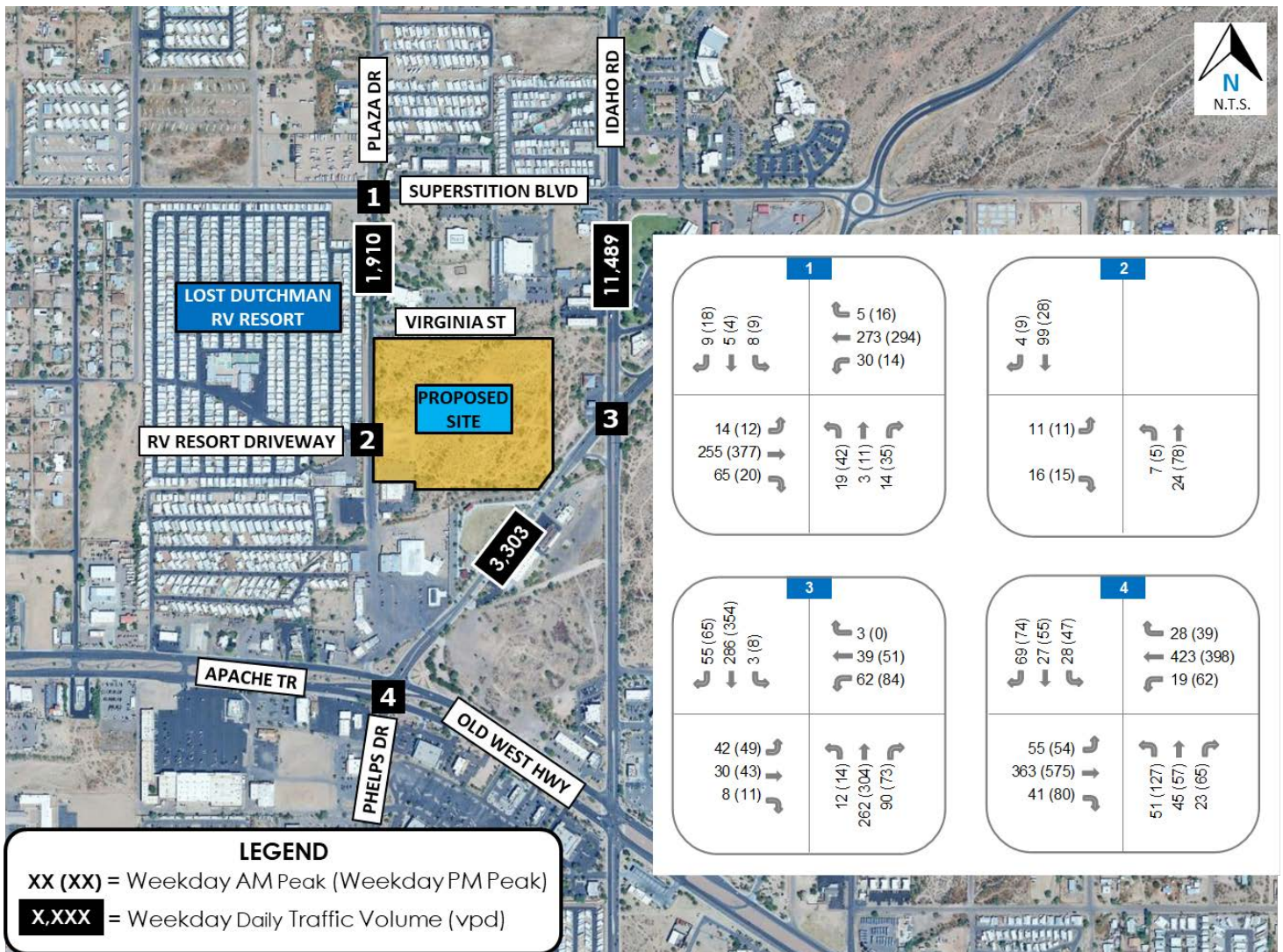


Figure 5: Existing Weekday Traffic Volumes (adjusted for seasonal variation with a 1.35 factor applied)

EXISTING LEVEL-OF-SERVICE ANALYSIS

The ability of a transportation system to transmit the transportation demand is characterized as its level-of-service (LOS). Level-of-service is a rating system from “A”, representing the best operation with the least delay, to “F”, representing the worst operation with the greatest delay. The appropriate reference for level-of-service operation is the *Highway Capacity Manual*, 7th Edition published by the Transportation Research Board in 2022.

This manual considers the average delay per vehicle as the measure to determine the level-of-service for both signalized and unsignalized intersections. For signalized intersections and for multi-way stop intersections, the delay and level-of-service are calculated for the intersection, each approach, and each turning movement. For unsignalized intersections the level-of-service is defined for each minor movement for two-way stop controls, and is not defined for the major street approaches or for the entire intersection **Figure 6** provides a diagram and **Table 2** lists the level-of-service criteria for both signalized and unsignalized intersections as stated in the *Highway Capacity Manual*.

Figure 6: Level-of-Service Criteria for Intersections

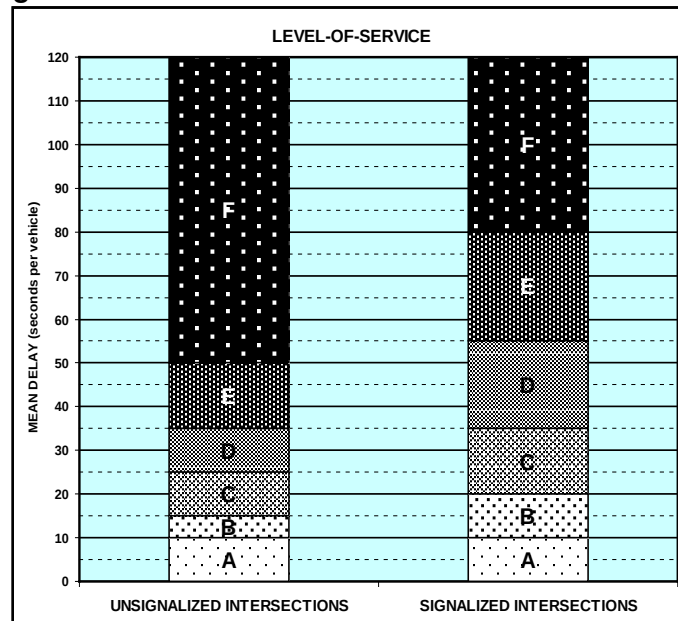


Table 2: Level-of-Service Criteria for Intersections

Level-of-Service	Average Delay (seconds per vehicle)	
	Unsignalized	Signalized
A	≤ 10	≤ 10
B	> 10 to 15	> 10 to 20
C	> 15 to 25	> 20 to 35
D	> 25 to 35	> 35 to 55
E	> 35 to 50	> 55 to 80
F	> 50	> 80

Level-of-service (LOS) analyses were prepared for the AM and PM peak hours for the study intersections utilizing Synchro software. The analysis worksheets are included in **Appendix D**. A summary of the level-of-service analysis for the existing conditions is provided in **Table 3**. The study intersections operate at acceptable levels of service with existing conditions.

Table 3: Existing Level-of-Service Analysis

Intersection	Traffic Control	Movement/ Approach	Existing Conditions			
			AM Peak Hour		PM Peak Hour	
			Average Delay (sec/veh)	LOS	Average Delay (sec/veh)	LOS
1						
Plaza Dr & Superstition Blvd	Minor Street Stop Control (NB/SB)	EB Left	7.9	A	8.0	A
		WB Left	8.1	A	8.2	A
		NB Left	14.8	B	17.1	C
		NB Thru/Right	10.5	B	11.9	B
		SB Left	14.1	B	15.6	C
		SB Thru/Right	11.8	B	10.9	B
2						
Plaza Dr & Existing RV Resort Driveway	Minor Street Stop Control (EB)	EB Approach	9.2	A	8.9	A
		NB Left	7.4	A	7.3	A
3						
Idaho Rd & Apache Tr	Traffic Signal	EB Left	8.2	A	32.8	C
		EB Thru	7.8	A	30.9	C
		EB Right	7.8	A	31.0	C
		EB Approach	8.0	A	31.9	C
		WB Left	8.3	A	34.6	C
		WB Right	7.8	A	30.7	C
		WB Right	7.8	A	0.0	A
		WB Approach	8.1	A	33.1	C
		NB Left	7.0	A	3.8	A
		NB Thru	6.1	A	3.2	A
		NB Right	6.1	A	3.1	A
		NB Approach	6.2	A	3.2	A
		SB Left	6.5	A	3.4	A
		SB Thru	6.6	A	3.5	A
		SB Right	6.7	A	3.5	A
		SB Approach	6.6	A	3.5	A
		INTERSECTION	6.7	A	9.9	A
4						
Apache Tr/Phelps Rd & Old West Hwy/Apache Tr	Traffic Signal	EB Left	38.8	D	42.0	D
		EB Thru	30.4	C	32.6	C
		EB Right	31.0	C	33.8	C
		EB Approach	31.6	C	33.7	C
		WB Left	48.1	D	54.1	D
		WB Right	32.9	C	30.0	C
		WB Right	34.0	C	30.4	C
		WB Approach	33.9	C	33.1	C
		NB Left	12.4	B	15.5	B
		NB Thru	13.9	B	18.1	B
		NB Right	14.0	B	18.3	B
		NB Approach	13.5	B	17.3	B
		SB Left	12.7	B	15.6	B
		SB Thru	14.4	B	18.3	B
		SB Right	14.4	B	18.5	B
		SB Approach	14.0	B	17.7	B
		INTERSECTION	28.6	C	29.1	C

PROPOSED DEVELOPMENT

SITE LOCATION, LAND USE, AND ACCESS

The multifamily residential development is proposed east of Plaza Drive between Superstition Boulevard and Apache Trail in Apache Junction, Arizona. The development is proposed with three-story buildings primarily consisting of one and two-bedroom units with a total of 336 dwelling units. The primary access is proposed on Apache Trail. A secondary access is proposed on Plaza Drive north of the existing RV Resort driveway and will be limited to a southbound left-in and westbound right-out only. Both entrances will be gate controlled. **Figure 7** depicts the site plan. The detailed site plan is included in **Appendix A**.

TRIP GENERATION

The trip generation for project was estimated utilizing the Institute of Transportation Engineers' (ITE) *Trip Generation Manual, 11th Edition*. The manual contains data collected by various transportation professionals for a wide range of different land uses. The data summarized in the manual include average rates and equations that have been established correlating the relationship between an independent variable that describes the development size and generated trips for each categorized land use. The manual provides information for daily and peak hour trips.

Table 4 summarizes the trip generation for the proposed development, and **Appendix E** provides the generated trip calculation.

Table 4: Trip Generation

DESCRIPTION OF LAND USE				GENERATED TRIPS						
				DAILY	AM PEAK HOUR			PM PEAK HOUR		
LAND USE	ITE LUC	SIZE		TOTAL	ENTER	EXIT	TOTAL	ENTER	EXIT	TOTAL
Multifamily Housing	220	336	DU	2,265	32	102	134	108	63	171

Full build-out of the proposed development is anticipated to generate 2,265 weekday daily trips (entering/exiting) with 134 trips (entering/exiting) during the AM peak hour and 171 trips (entering/exiting) during the PM peak hour.

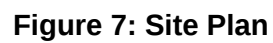
TRIP GENERATION COMPARISON

The existing B-3 zoning allows commercial uses. A commercial building size of 202,000 square feet (25% FAR) was utilized to compare potential trips generated with the existing zoning to the proposed development.

Table 5 summarizes the trip generation comparison to a shopping center use. When compared to a shopping center use allowed under B-3 zoning, the proposed multi-family development generates 5,211 fewer daily trips, 36 fewer AM peak hour trips, and 516 fewer PM peak hour trips. When compared to an office use, the proposed multi-family development generates 126 more daily trips, 172 fewer AM peak hour trips and 127 fewer evening peak hour trips.

Table 5: Trip Generation Comparison – Shopping Center

DESCRIPTION OF LAND USE				VEHICLE GENERATED TRIPS						
				Daily	AM Peak Hour			PM Peak Hour		
Land Use	ITE LUC	SIZE		Total	Enter	Exit	Total	Enter	Exit	Total
Multifamily Housing	220	336	DU	2,265	32	102	134	108	63	171
Shopping Center	820	202,000	SF	7,476	105	65	170	330	357	687
Difference				-5,211	-73	37	-36	-222	-294	-516



TRIP DISTRIBUTION AND ASSIGNMENT

The generated trips for the proposed development have been distributed and assigned to the surrounding street system to estimate future traffic from the development. The distribution was based on existing traffic patterns and employment centers. The distribution percentages utilized in this analysis depicted in **Figure 8**. Utilizing the trip generation and distribution, new trips from the proposed development were assigned to the study intersections and are depicted in **Figure 9**.

FUTURE TRAFFIC

An additional annual 5% growth rate was used to estimate the growth of ambient background traffic in the horizon years. The peak hour background without site traffic volumes are depicted in **Figure 10** for the 2026 horizon year and in **Figure 11** for the 2031 horizon year. Full build-out of the site was assumed for opening year in this analysis, which is a conservative estimate. The total weekday traffic volumes with the site are depicted in **Figure 12** and **Figure 13** for horizon years 2026 and 2031 respectively.

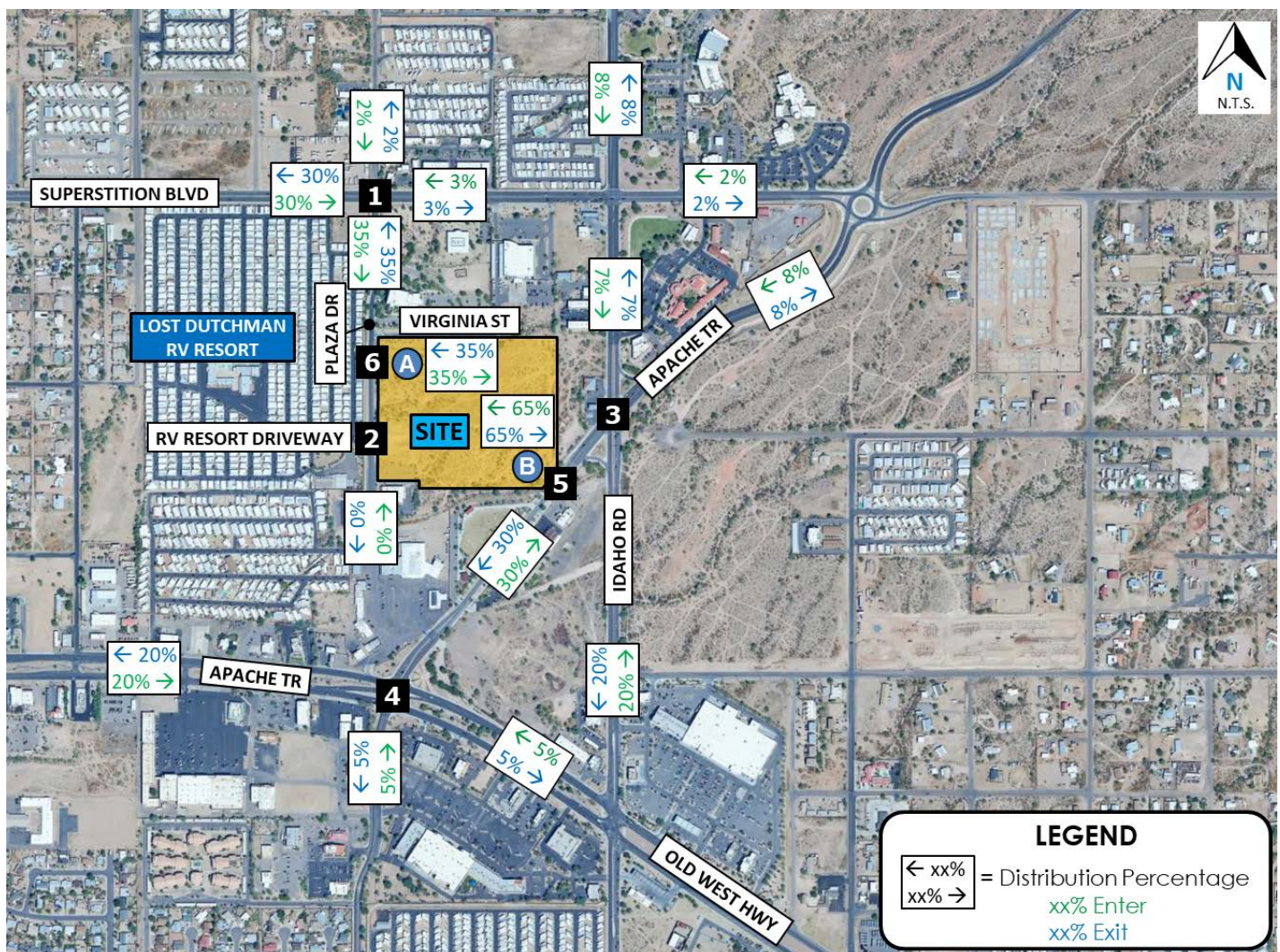


Figure 8: Multifamily Residential Trip Distribution

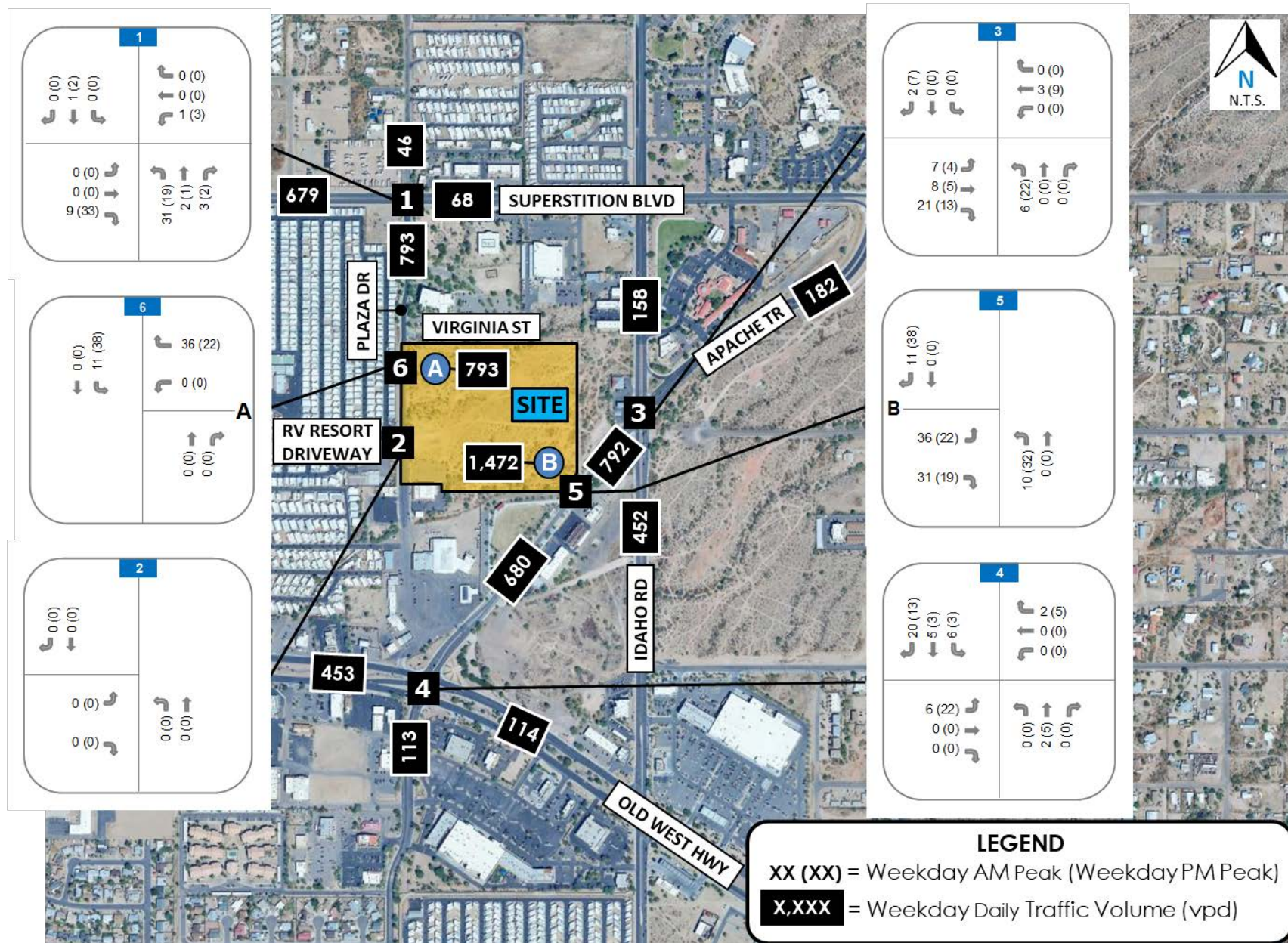


Figure 9: Site Weekday Traffic Volumes

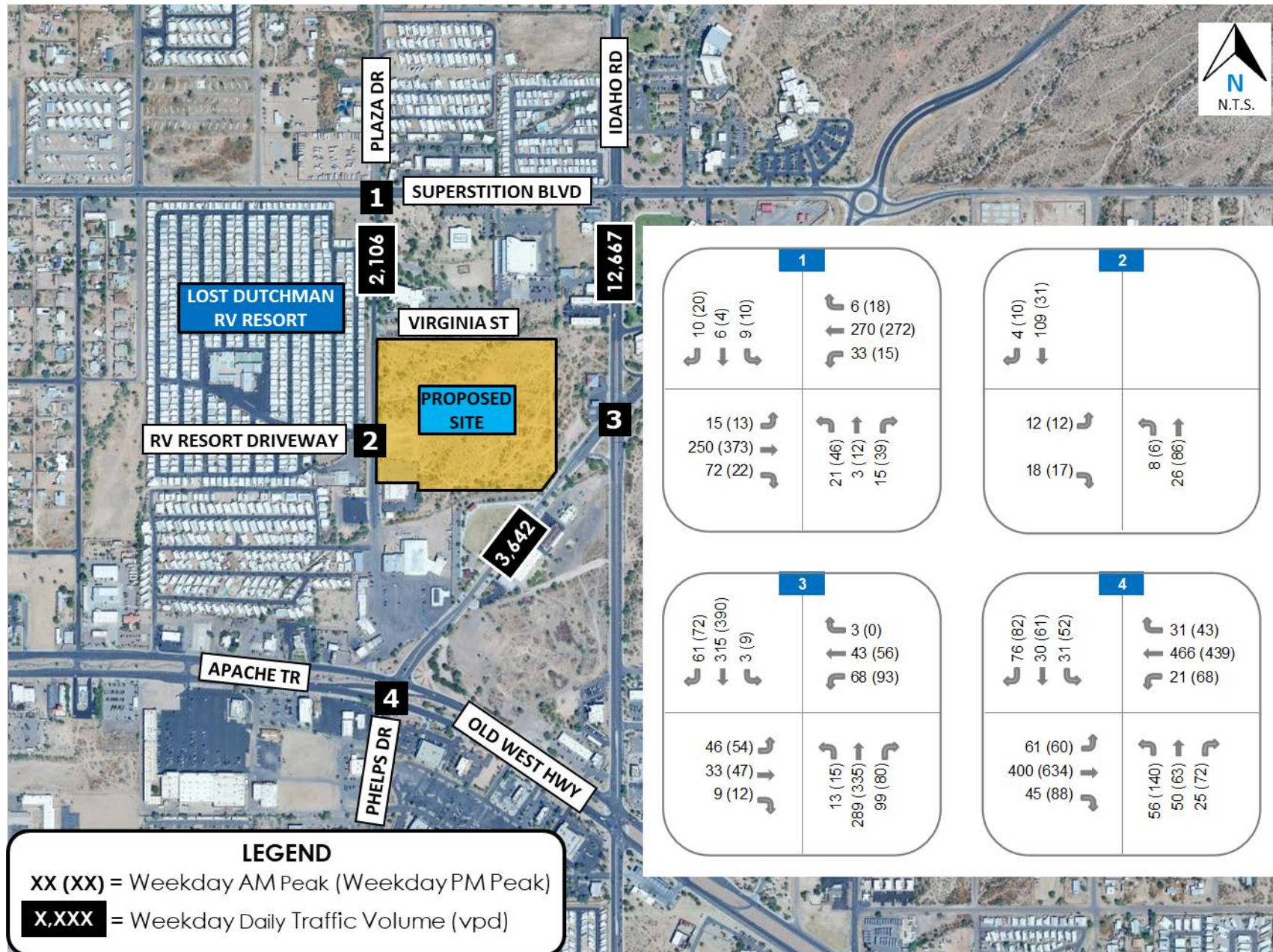


Figure 10: 2026 Background without Site Weekday Traffic Volumes

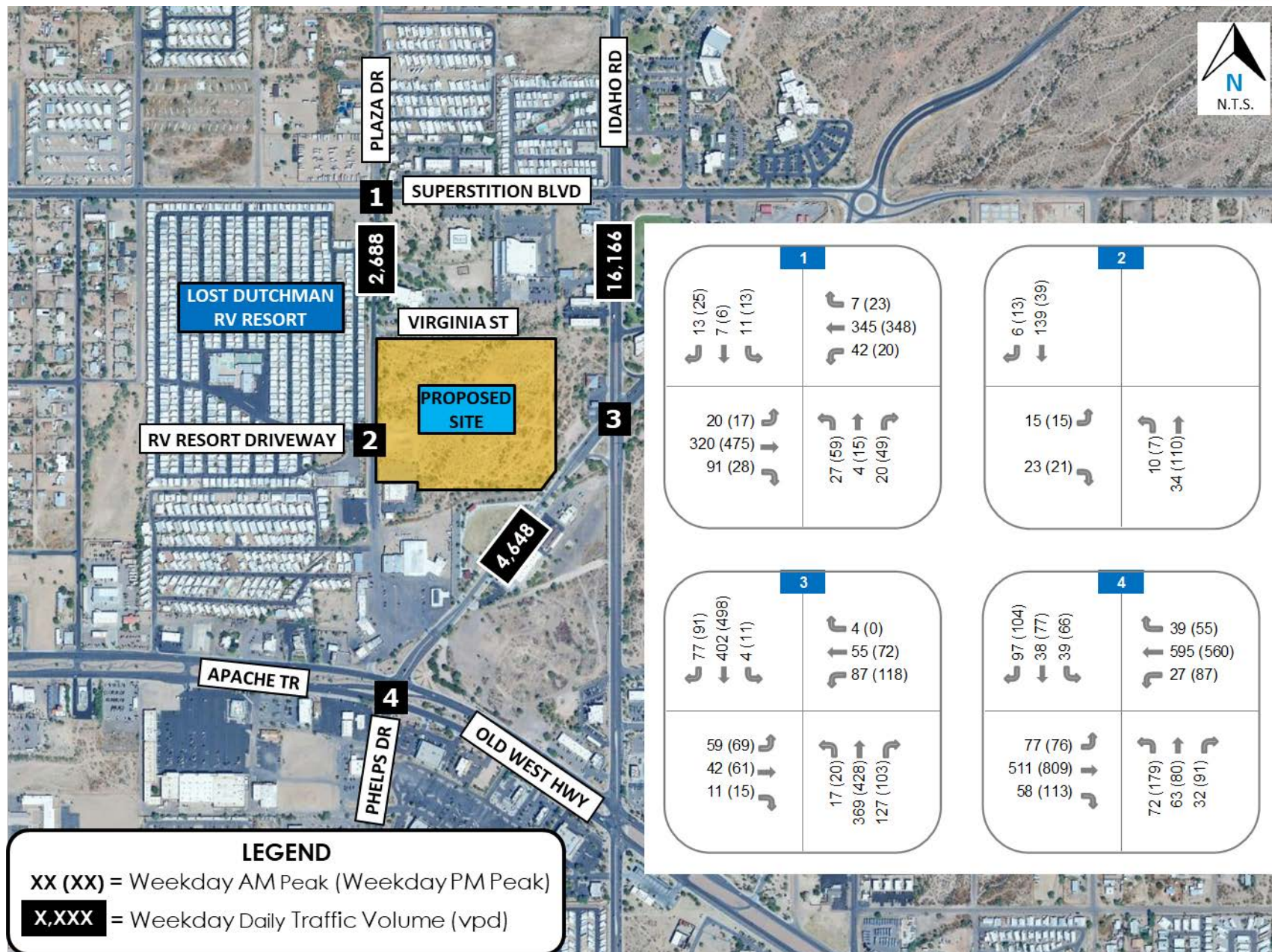


Figure 11: 2031 Background without Site Weekday Traffic Volumes

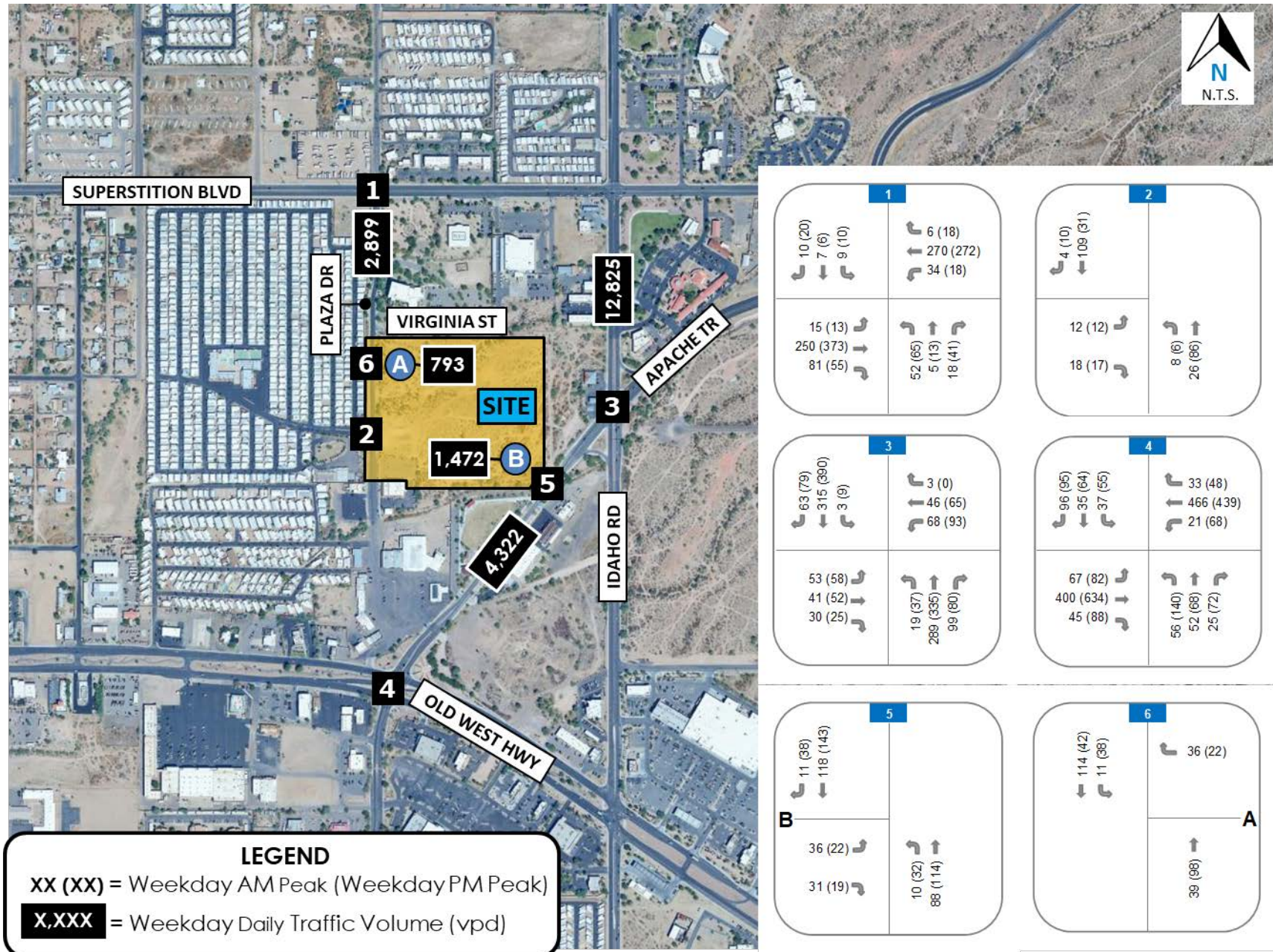


Figure 12: 2026 Total (Background with Site) Traffic Volumes

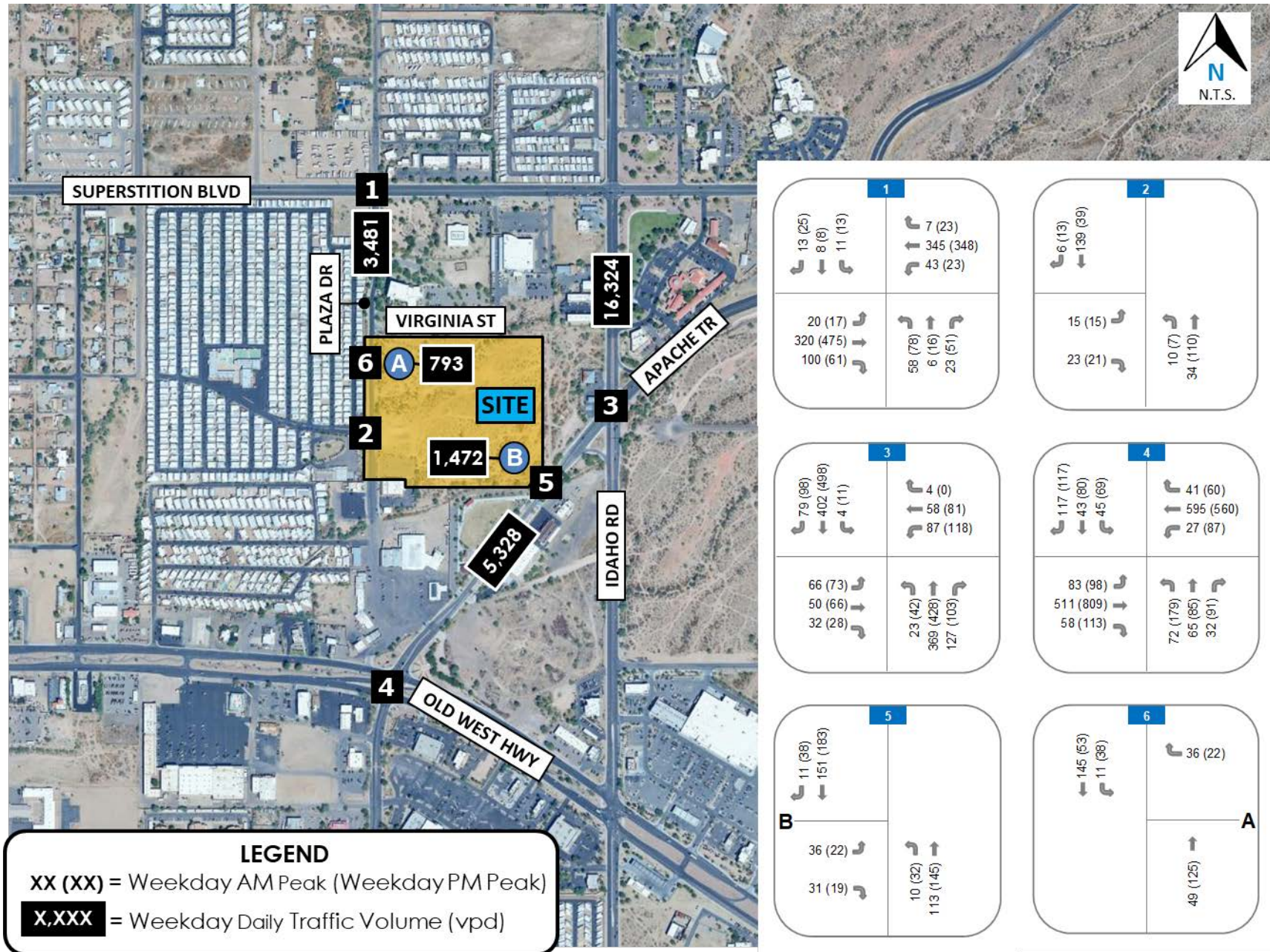


Figure 13: 2031 Total (Background with Site) Traffic Volumes

FUTURE TRAFFIC ANALYSES

FUTURE LEVEL-OF-SERVICE

The level-of-service for the future 2026 and 2031 horizon years was analyzed for the study intersections. Synchro software was utilized to calculate the average delay and level-of-service. The input and output for these analyses are provided in **Appendix F** and **Appendix G**. **Table 6** and **Table 7** summarize the LOS for horizon years 2026 and 2031 respectively.

With trips from the proposed development, the study intersections will continue to operate at acceptable levels of service in both horizon years 2026 and 2031 with the existing lane configuration and traffic control.

The two closest intersections to the proposed development's driveways are Plaza Drive/Superstition Drive and Apache Trail/Idaho Road. With the seasonal factor of 1.35 applied, the added site traffic is projected to add three seconds of delay to the northbound approach of the Plaza Drive/Superstition Boulevard intersection during the AM multi-family and PM multi-family peak hours. At the Apache Trail/Idaho Road intersection and with the seasonal factor of 1.35 applied, site traffic is expected to add 0.1 and 0.3 seconds of delay to the overall LOS during the AM multi-family peak hour and PM multi-family peak hour respectively.

TRANSPORTATION IMPROVEMENTS

As part of the proposed development, half-street improvements will be provided on Apache Trail adjacent to the property. Half-street improvements will also be provided on Plaza Drive, which will include the provision of angled on-street parking in the northbound direction.

SITE ACCESS

A full access driveway is proposed on Apache Trail. A secondary driveway is proposed on Plaza Drive north of the existing RV Resort driveway and will be limited to a southbound left-in and westbound right-out only. The driveways will be controlled by a stop sign. Both Plaza Drive and Apache Trail are low volume roadways with a projected daily traffic volume less than 3,500 vpd on Plaza Drive and less than 5,400 vpd on Apache Trail. The posted speed limit on Apache Trail is 15 mph. A speed limit sign of 15 mph is provided on Plaza Drive for the existing Lost Dutchman RV Resort.

AUXILIARY LANES

When warranted, turn lanes permit separation of conflicting traffic movements and remove the slower turning traffic from the through traffic, thus improving capacity and reducing rear-end crashes. Determining the need for turn lanes is a key element of traffic impact studies.

RIGHT-TURN DECELERATION LANE

The *2024 Pinal County Subdivision & Infrastructure Design Manual* states that right-turn deceleration lanes are required when both of the following factors are determined to apply:

- A. The 85th percentile traffic speed on the street is at least 35 miles per hour or 45 miles per hour for a two lane (one lane each direction) roadway.
- B. At least 20 vehicles will be making right turns into the access way and 100 vehicle directional traffic during a one-hour period.

The posted speed limit on Apache Trail is 15 mph, and the southbound right-turn volume at the site driveway is projected to be 11 vph during the AM peak hour and 38 vph during the PM peak hour. The directional DHV in the southbound direction is 151 vph during the AM peak hour and 183 vph during the PM peak hour. Due to the posted speed of 15 mph, a right-turn deceleration lane is not warranted at the site access on Apache Trail.

The site driveway on Plaza Drive is proposed as southbound left-in and westbound right-out only.

LEFT-TURN LANE

The posted speed limit on Apache Trail is 15 mph, and the northbound left-turn volume at the site driveway is projected to be 10 vph during the AM peak hour and 32 vph during the PM peak hour. The directional DHV in the northbound direction is 113 vph during the AM peak hour and 145 vph during the PM peak hour. Due to the posted speed of 15 mph, a separate left-turn lane is not warranted at the site driveway.

The posted speed limit on Plaza Drive is 15 mph, and the southbound left-turn volume at the site driveway is projected to be 11 vph during the AM peak hour and 38 vph during the PM peak hour. The directional DHV in the southbound direction is 145 vph during the AM peak hour and 53 vph during the PM peak hour. Due to the posted speed of 15 mph, a separate left-turn lane is not warranted at the site driveway.

Table 6: Level-of-Service Summary – Horizon Year 2026

Intersection	Traffic Control	Movement/ Approach	2026 Background				2026 Background + Site Full Build-out			
			AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour	
			Delay (Sec/Veh)	LOS	Delay (Sec/Veh)	LOS	Delay (Sec/Veh)	LOS	Delay (Sec/Veh)	LOS
1 Plaza Dr & Superstition Blvd	Minor Street Stop Control (NB/SB)									
		EB Left	7.9	A	7.9	A	7.9	A	7.9	A
		WB Left	8.1	A	8.2	A	8.1	A	8.4	A
		NB Left	15.0	B	17.1	C	16.2	C	18.9	C
		NB Thru/Right	10.5	B	11.8	B	11.0	B	12.2	B
		NB Approach	12.9	B	14.3	B	14.6	B	15.9	C
		SB Left	14.2	B	15.3	C	14.3	B	15.6	C
		SB Thru/Right	12.0	B	10.6	B	12.4	B	11.4	B
		SB Approach	12.8	B	12.0	B	13.1	B	12.6	B
2 Plaza Dr & RV Resort Driveway	Minor Street Stop Control (EB)									
		EB Approach	9.2	A	9.0	A	9.2	A	9.0	A
		NB Left	7.5	A	7.3	A	7.5	A	7.3	A
3 Idaho Rd & Apache Tr	Traffic Signal									
		EB Left	8.4	A	32.8	C	8.5	A	32.8	C
		EB Thru	8.0	A	30.6	C	8.1	A	30.6	C
		EB Right	8.0	A	30.7	C	8.2	A	30.7	C
		EB Approach	8.2	A	31.7	C	8.3	A	31.6	C
		WB Left	8.6	A	34.7	C	8.8	A	34.9	C
		WB Thru/Right	8.0	A	30.5	C	8.0	A	30.3	C
		WB Right	8.0	A	0.0	A	8.0	A	30.3	C
		WB Approach	8.4	A	33.1	C	8.5	A	33.0	C
		NB Left	7.2	A	4.2	A	7.4	A	4.7	A
		NB Thru/Right	6.2	A	3.4	A	6.3	A	3.6	A
		NB Right	6.2	A	3.3	A	6.3	A	3.5	A
		NB Approach	6.2	A	3.4	A	6.3	A	3.7	A
		SB Left	6.6	A	3.8	A	6.7	A	4.0	A
		SB Thru/Right	6.7	A	3.8	A	6.8	A	4.1	A
		SB Right	6.7	A	3.9	A	6.8	A	4.1	A
		SB Approach	6.7	A	3.8	A	6.8	A	4.1	A
		INTERSECTION	6.8	A	10.2	B	7.0	A	10.7	B
4 Apache Tr/Phelps Rd & Old West Hwy/Apache Tr	Traffic Signal									
		EB Left	39.7	D	43.5	D	40.0	D	44.0	D
		EB Thru	30.4	C	32.9	C	30.5	C	33.0	C
		EB Right	31.1	C	34.1	C	31.1	C	34.1	C
		EB Approach	31.7	C	34.1	C	31.9	C	34.5	C
		WB Left	48.8	D	54.0	D	49.1	D	54.0	D
		WB Right	33.0	C	29.7	C	33.2	C	30.2	C
		WB Right	34.1	C	30.1	C	34.3	C	30.7	C
		WB Approach	34.0	C	32.8	C	34.2	C	33.3	C
		NB Left	13.0	B	16.7	B	13.1	B	16.7	B
		NB Thru	14.7	B	19.6	B	15.0	B	19.6	B
		NB Right	14.8	B	19.8	B	15.1	B	19.8	B
		NB Approach	14.2	B	18.7	B	14.4	B	18.8	B
		SB Left	13.2	B	16.8	B	13.2	B	16.8	B
		SB Thru	15.1	B	19.7	B	15.4	B	20.0	B
		SB Right	15.1	B	20.0	B	15.4	B	20.2	C
		SB Approach	14.7	B	19.1	B	15.0	B	19.3	B
		INTERSECTION	28.9	C	28.3	C	28.7	C	29.8	C
5 Apache Tr & Site Access B	Minor Street Stop Control (EB)									
		EB Left	-		-		10.2	B	11.1	B
		EB Right	-		-		9.1	A	9.3	A
		EB Approach	-		-		9.7	A	10.3	B
		NB Left	-		-		7.5	A	7.7	A
6 Plaza Dr & Site Access A	Minor Street Stop Control (WB)									
		WB Right	-		-		8.6	A	8.9	A
		SB Left	-		-		7.3	A	7.5	A

Table 7: Level-of-Service Summary – Horizon Year 2031

Intersection	Traffic Control	Movement/ Approach	2031 Background				2031 Background + Site Full Build-out			
			AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour	
			Delay (Sec/Veh)	LOS	Delay (Sec/Veh)	LOS	Delay (Sec/Veh)	LOS	Delay (Sec/Veh)	LOS
1	Plaza Dr & Superstition Blvd	Minor Street Stop Control (NB/SB)								
EB Left			8.1	A	8.2	A	8.1	A	8.2	A
WB Left			8.4	A	8.6	A	8.4	A	8.7	A
NB Left			19.1	C	24.1	C	21.5	C	28.1	D
NB Thru/Right			11.5	B	13.8	B	12.1	B	14.2	B
NB Approach			15.5	C	18.7	C	18.4	C	21.7	C
SB Left			17.4	C	19.5	C	17.6	C	20.0	C
SB Thru/Right			13.6	B	12.1	B	14.1	B	13.1	B
		SB Approach	14.9	B	14.3	B	15.3	C	15.1	C
2	Plaza Dr & RV Resort Driveway	Minor Street Stop Control (EB)								
EB Approach			9.5	A	9.1	A	9.5	A	9.1	A
NB Left			7.6	A	7.3	A	7.6	A	7.3	A
3	Idaho Rd & Apache Tr	Traffic Signal								
EB Left			9.3	A	32.6	C	9.4	A	32.6	C
EB Thru			8.7	A	29.8	C	8.9	A	29.7	C
EB Right			8.7	A	29.9	C	8.9	A	29.9	C
EB Approach			9.0	A	31.2	C	9.1	A	31.0	C
WB Left			9.5	A	35.0	C	9.8	A	35.2	D
WB Thru/Right			8.7	A	29.7	C	8.8	A	29.5	C
WB Right			8.7	A	29.7	C	8.8	A	29.5	C
WB Approach			9.2	A	33.0	C	9.4	A	32.9	C
NB Left			7.6	A	5.7	A	7.7	A	6.3	A
NB Thru/Right			6.2	A	4.3	A	6.2	A	4.5	A
NB Right			6.1	A	4.1	A	6.2	A	4.3	A
NB Approach			6.2	A	4.3	A	6.3	A	4.6	A
SB Left			6.8	A	4.8	A	6.8	A	5.1	A
SB Thru/Right			6.8	A	4.9	A	6.8	A	5.2	A
SB Right			6.8	A	4.9	A	6.8	A	5.2	A
		SB Approach	6.8	A	4.9	A	6.8	A	5.2	A
		INTERSECTION	7.0	A	10.8	B	7.2	A	11.3	B
4	Apache Tr/Phelps Rd & Old West Hwy/Apache Tr	Traffic Signal								
EB Left			42.3	D	46.9	D	42.8	D	47.8	D
EB Thru			30.5	C	33.1	C	30.6	C	33.2	C
EB Right			31.2	C	34.4	C	31.2	C	34.5	C
EB Approach			32.1	C	34.6	C	32.3	C	35.0	C
WB Left			50.9	D	53.5	D	51.2	D	53.5	D
WB Right			33.1	C	28.0	C	33.3	C	28.4	C
WB Right			34.1	C	28.4	C	34.3	C	28.8	C
WB Approach			34.2	C	31.3	C	34.4	C	31.7	C
NB Left			14.8	B	20.6	C	14.9	B	20.6	C
NB Thru			16.9	B	24.3	C	17.2	B	24.4	C
NB Right			17.0	B	24.6	C	17.3	B	24.7	C
NB Approach			16.3	B	23.3	C	16.5	B	23.3	C
SB Left			15.0	B	21.3	C	15.0	B	21.3	C
SB Thru			17.4	B	25.2	C	17.7	B	25.5	C
SB Right			17.4	B	25.5	C	17.7	B	25.8	C
		SB Approach	16.9	B	24.3	C	17.1	B	24.6	C
		INTERSECTION	29.6	C	30.6	C	29.5	C	30.9	C
5	Apache Tr & Site Access B	Minor Street Stop Control (EB)	13.9							
EB Left			-		-		10.7	B	11.8	B
EB Right			-		-		9.3	A	9.5	A
EB Approach			-		-		10.1	B	10.7	B
NB Left			-		-		7.6	A	7.8	A
6	Plaza Dr & Site Access A	Minor Street Stop Control (WB)								
WB Right			-		-		8.7	A	9.0	A
SB Left			-		-		7.3	A	7.6	A

PRINCIPAL FINDINGS AND RECOMMENDATIONS

TRIP GENERATION

Full build-out of the proposed development is anticipated to generate 2,265 weekday daily trips (entering/exiting) with 134 trips (entering/exiting) during the AM peak hour and 171 trips (entering/exiting) during the PM peak hour. Existing traffic counts reveal that the existing peak hour of traffic at the study intersections generally occurs at 12:00 PM, which differs from the morning and evening peak hour periods of multi-family residential land uses.

When compared to a shopping center use allowed under B-3 zoning, the proposed multi-family development generates 5,211 fewer daily trips, 36 fewer AM peak hour trips, and 516 fewer PM peak hour trips.

SEASONAL FACTOR

The City of Apache Junction annually collects traffic volumes in February or March and is available to the public. Additionally, both the Arizona Department of Transportation (ADOT) and Maricopa Association of Governments (MAG) regularly collect and compile traffic data on many roadways within their boundaries. At the applicant's request, this report has been revised to compare all recent historical traffic volumes near the proposed site from these agencies and compare them to the counts collected in September 2024 as part of this study.

There were several available counts recorded near the proposed site during both the higher traffic winter months and lower traffic summer months. Based on the evaluation of the relevant count data, an approximate 32% difference was observed between the peak and non-peak seasons. To provide a conservative evaluation, a 35% growth factor was utilized. Therefore, in addition to the typical annual background growth rate, a seasonal factor of 1.35 was applied to the existing traffic volumes collected in September for use in this analysis.

LEVEL-OF-SERVICE

The study intersections operate at acceptable levels of service with existing conditions. With trips from the proposed development, the study intersections will continue to operate at acceptable levels of service in both horizon years 2026 and 2031 with the existing lane configurations and traffic control.

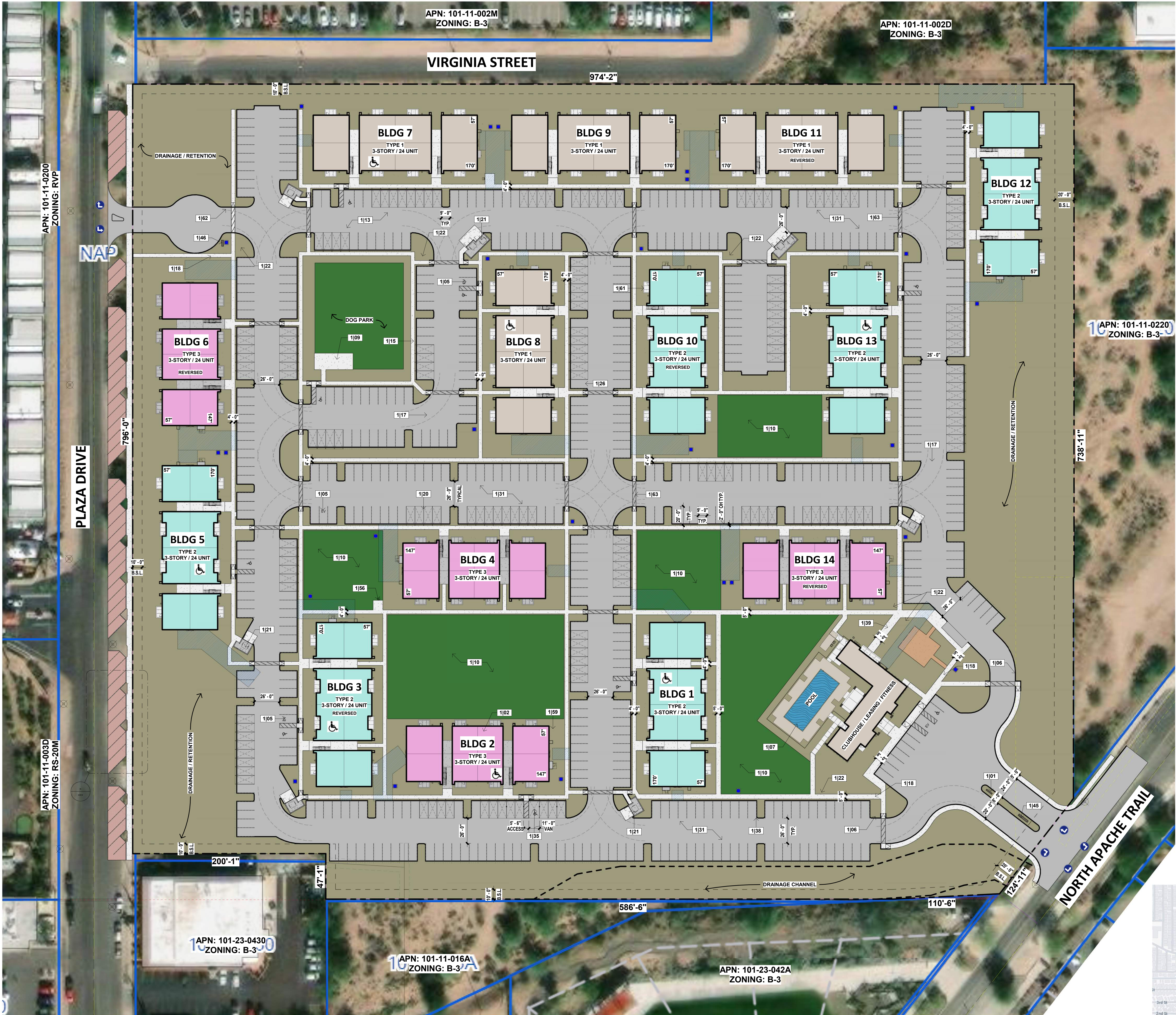
The two closest intersections to the proposed development's driveways are Plaza Drive / Superstition Drive and Apache Trail / Idaho Road. With the seasonal factor of 1.35 applied, the added site traffic is projected to add 3.0 seconds of delay to the northbound approach of the Plaza Drive / Superstition Boulevard intersection during the AM and PM multi-family peak hours. At the Apache Trail / Idaho Road intersection, with the seasonal factor of 1.35 applied, site traffic is expected to add 0.2 and 0.5 seconds of delay to the overall intersection during the AM and PM multi-family peak hours, respectively.

SITE ACCESS

A full access driveway is proposed on Apache Trail. A secondary driveway is proposed on Plaza Drive north of the existing RV Resort driveway and will be limited to a southbound left-in and westbound right-out only. The driveways will be controlled by a stop sign. Both Plaza Drive and Apache Trail are low volume roadways with a projected daily traffic volume less than 3,500 vpd on Plaza Drive and less than 5,400 vpd on Apache Trail. The posted speed limit on Apache Trail is 15 mph. A speed limit sign of 15 mph is provided on Plaza Drive for the existing Lost Dutchman RV Resort. As part of the proposed development, half-street improvements will be provided on Apache Trail adjacent to the property. Half-street improvements will also be provided on Plaza Drive, which will include the provision of angled on-street parking in the northbound direction.

APPENDICES

APPENDIX A: Site Plan



DEVELOPER:
WOLFF ENTERPRISES III, LLC
4115 NORTH 15TH AVENUE
SCOTTSDALE, ARIZONA 85015
CONTACT: DEREK CAYTON
PHONE: (480) 236-0998
EMAIL: dcayton@awolff.com

ARCHITECT:
STUDIO 15 ARCHITECTURE, INC.
4115 NORTH 15TH AVENUE
PHOENIX, ARIZONA 85015
CONTACT: REX BOYES
SHAN HARRIS
PHONE: (602) 586-1190
EMAIL: rex@studio15a.com
shan@studio15a.com

REQUIRED PARKING:
MULTIFAMILY
ONE BEDROOM 48X1.5 = 72 STALLS
TWO BEDROOM 288X2 = 576 STALLS
GUEST 1 SPACE PER 0.30 UNITS = 101 STALLS

TOTAL REQUIRED: 749 STALLS
2:22 PARKING RATIO REQUIRED

PARKING REQUIRED WITH 10% REDUCTION WITH ZONING
ADMINISTRATOR REVIEW PER 1-7.4. 90% OF 749 = 674.1 STALLS

ON-SITE PARKING PROVIDED
CARPORT 162 STALLS
CARPORT (ACCESSIBLE) 1 STALLS
EV CHARGING 4 STALLS
SURFACE 509 STALLS
TOTAL PARKING (ACCESSIBLE) 684 STALLS

8.7% REDUCTION REQUEST

OFF-SITE PARKING: 45 STALLS

PARKING RATIO:
729 STALLS / 336 TOTAL D.U. = 2.17 (INCLUDES OFF-SITE)
729 - 45 = 684 STALLS / 336 TOTAL D.U. = 2.04

BIKE PARKING REQUIRED: 6 SPACES + 2 FOR EVERY 40 STALLS
749 STALL / 40 * 2 + 6 = 44 SPACES REQUIRED

- KEYNOTES
- 1101

SITE DIRECTORY UNDER SEPARATE PERMIT - REFER TO SIGN PACKAGE / BUTTERFLY CALL BOX ON TUBE STEEL PEDESTAL WITH CONCRETE FOOTING
- 1102

INDICATES 1ST FLOOR TYPE A UNIT - TYPICAL
- 1105

INDICATES SINGLE EV CHARGING STATION - TYPICAL
- 1106

26 FOOT WIDE VEHICULAR GATE W/ KNOX BOX FOR EMERGENCY FIRE ACCESS - UNDER SEPARATE PERMIT. AT MAIN ENTRANCE GATE TO BE PROVIDED WITH OPTICOM FOR FIRE DEPARTMENT ACCESS
- 1107

POOL AREA - REFER TO LANDSCAPE PLANS
- 1109

DOG PARK - REFER TO LANDSCAPE PLANS
- 1110

OPEN SPACE
- 1113

26' WIDE DRIVE AISLE TYPICAL U.N.O.
- 1115

ALL SIDEWALKS ARE MINIMUM 5' WIDE TYPICAL U.N.O.
- 1117

INDICATES FIRE TURNING RADIUS TYPICAL - INSIDE 40', OUTSIDE 56'
- 1118

PEDESTRIAN GATE - REFER TO LANDSCAPE - TYPICAL
- 1120

INDICATES COVERED PARKING UNDER SEPARATE PERMIT - TYPICAL
- 1121

INDICATES REFUSE ENCLOSURE - TYPICAL
- 1122

INDICATES BIKE RACK TYPICAL - REFER TO LANDSCAPE
- 1126

PARKING STALL 9' WIDE X 18' LONG - TYPICAL U.N.O.
- 1131

ASPHALT DRIVE - REFER TO CIVIL
- 1135

INDICATES ACCESSIBLE PARKING STALL - TYPICAL
- 1138

CONCRETE CURB - REFER TO CIVIL
- 1139

CONCRETE CART DRIVE - REFER TO CIVIL
- 1145

MONUMENT SIGN LOCATION
- 1146

SITE DIRECTORY UNDER SEPARATE PERMIT - REFER TO SIGN PACKAGE
- 1156

BBQ TO BE MINIMUM 10 FOOT CLEAR FROM ANY COMBUSTIBLE STRUCTURE. TYPICAL ALL LOCATIONS, SEE LANDSCAPE PLAN FOR LOCATIONS
- 1159

GROUND MOUNTED MECHANICAL UNITS TO BE SCREENED WITH MASONRY SCREEN WALL
- 1161

PRIVATE PATIO - SCREEN WALL AROUND PATIOS AT FRONT OF BUILDINGS
- 1162

26 FOOT WIDE VEHICULAR GATE W/ KNOX BOX FOR EMERGENCY FIRE ACCESS - GATE FOR RESIDENCE ENTRANCE AND EXIT ONLY, NO GUEST ENTRANCE - UNDER SEPARATE PERMIT
- 1163

CONTRASTING COLOR PAINTED ASPHALT CROSS WALK, MATCH SIDEWALK WIDTH.

UNIT MIX				
UNIT TYPE	# OF BEDROOMS	UNIT S.F.	# OF UNITS	PERCENTAGE
UNIT A1	1	750 SF	48	36,000 SF 14%
			48	36,000 SF 14%
UNIT B1	2	1,068 SF	216	236,688 SF 64%
UNIT B2	2	1,012 SF	72	72,864 SF 21%
Grand total			336	339,552 SF 100%

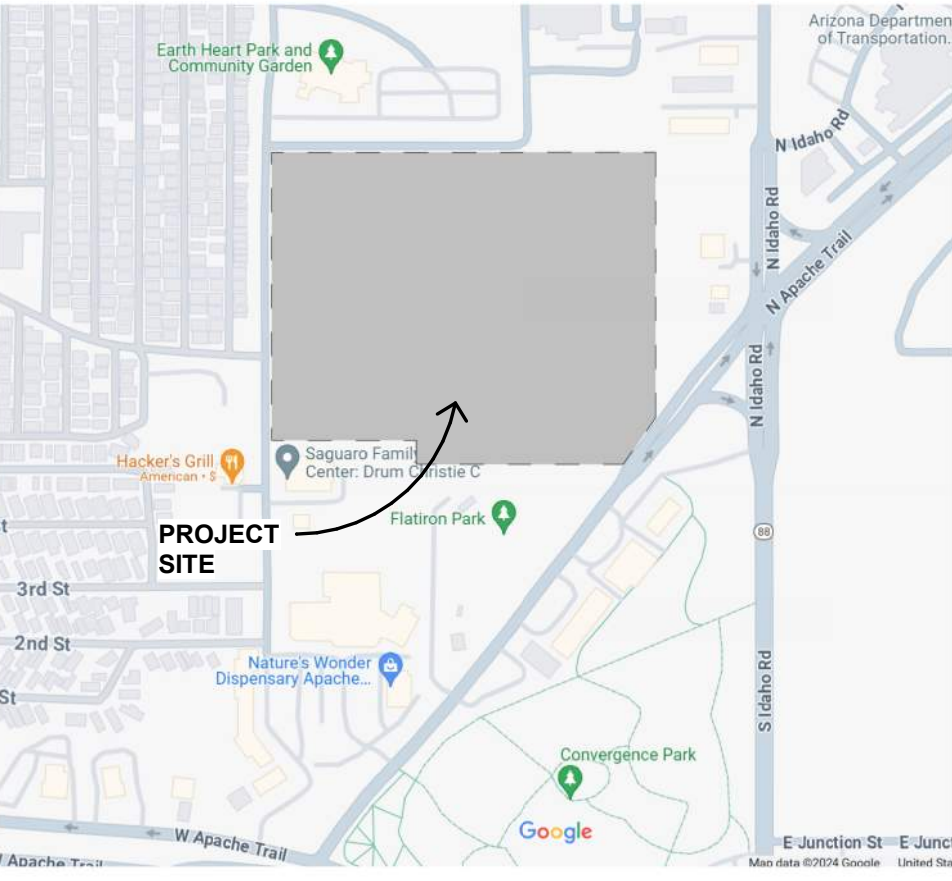
CODES

2018 INTERNATIONAL BUILDING CODE (IBC)
2018 INTERNATIONAL RESIDENTIAL CODE (IRC)
2018 INTERNATIONAL MECHANICAL CODE (IMC)
2018 UNIFORM MECHANICAL CODE (UPC)
2018 INTERNATIONAL PLUMBING CODE (IPC)
2018 UNIFORM PLUMBING CODE (UPC)
2018 INTERNATIONAL ENERGY CONSERVATION CODE (IECC)
2017 NATIONAL ELECTRICAL CODE (NEC)
2010 AMERICANS WITH DISABILITIES ACT STANDARDS FOR ACCESSIBLE DESIGN
ICC/ANSI A117.1-2009 ACCESSIBLE AND USABLE BUILDINGS AND FACILITIES
APPROVED ALTERNATIVE METHODS OF CONSTRUCTION PERMIT # B-2025-481

W

The Wolff Company

Apache Trail



PRELIMINARY
NOT FOR
CONSTRUCTION

04/01/2025

DRAWINGS, SPECIFICATIONS AND OTHER DOCUMENTS PREPARED BY STUDIO 15 ARCHITECTURE, INC. ARE INSTRUMENTS OF SERVICE AND ARE FOR THE CLIENT'S USE ONLY. THESE DOCUMENTS SHALL NOT BE USED BY OTHERS OR FOR OTHER PROJECTS WITHOUT THE WRITTEN CONSENT OF STUDIO 15 ARCHITECTURE, INC. THESE DOCUMENTS SHALL REMAIN THE PROPERTY OF STUDIO 15 ARCHITECTURE, INC. AND SHALL BE RETURNED TO STUDIO 15 ARCHITECTURE, INC. UPON COMPLETION OF THE PROJECT.

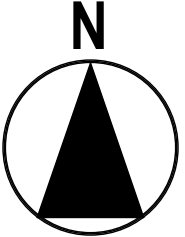
DATE: April 01, 2025 JOB NO.: 24-022
CURRENT REVISION: SHEET:

A1.10

Site Plan

Studio 15 Architecture, Inc. info@studio15a.com
4115 North 15th Avenue • Phoenix, Arizona 85015 • 602.586.1190

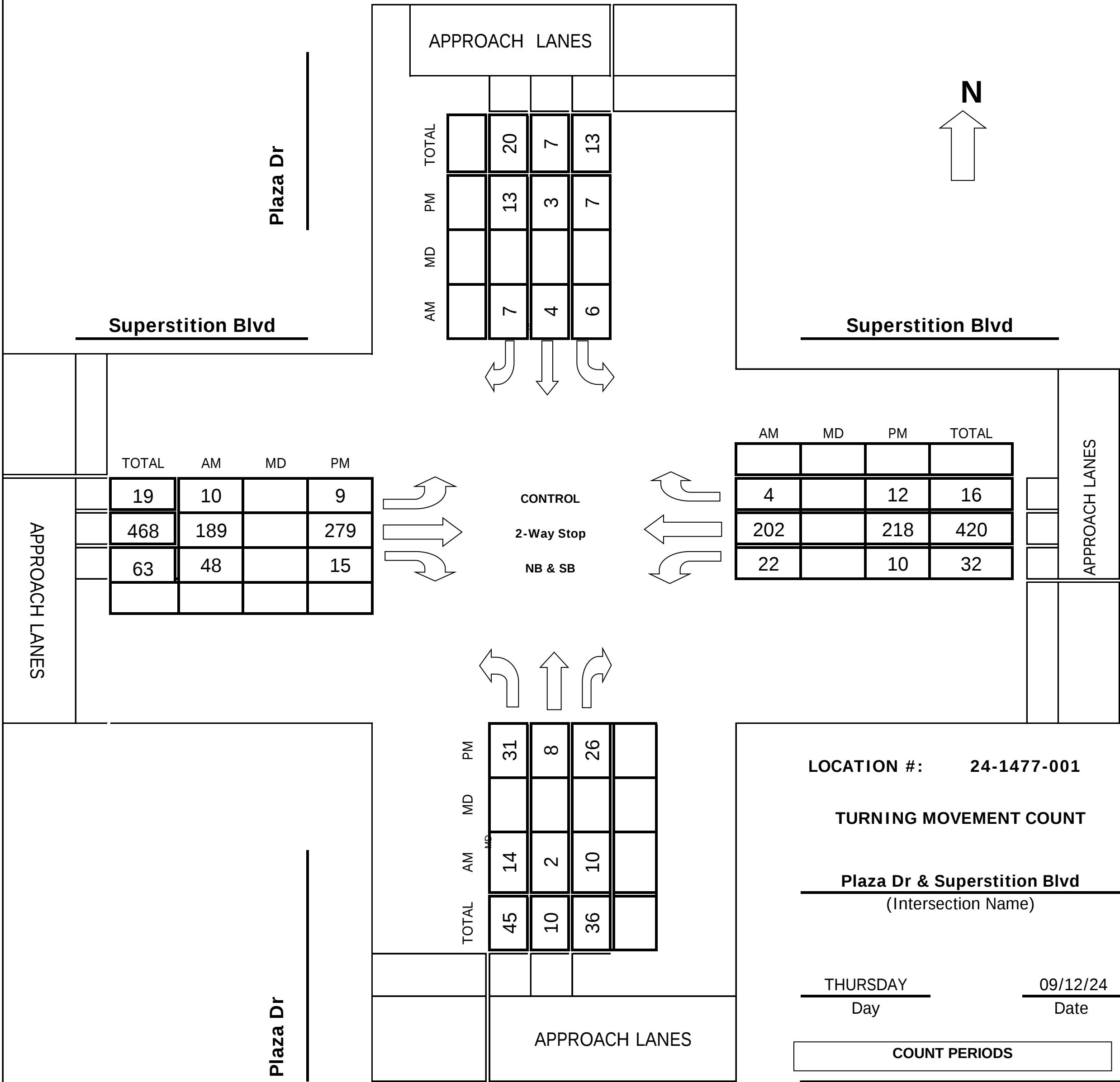
SITE PLAN
1" = 40'-0"



APPENDIX B: Existing Traffic Counts

Project #: 24-1477-001

TMC SUMMARY OF Plaza Dr & Superstition Blvd



LOCATION #: 24-1477-001

TURNING MOVEMENT COUNT

Plaza Dr & Superstition Blvd
(Intersection Name)

THURSDAY 09/12/24
Day Date

COUNT PERIODS			
AM	700AM	-	900AM
NOON		-	
PM	400PM	-	600PM

AM PEAK HOUR 730 AM

NOON PEAK HOUR

PM PEAK HOUR 430 PM

Intersection Turning Movement

Prepared by:



FIELD DATA SERVICES OF ARIZONA, INC.
520.316.6745



veracitytrafficgroup

N-S STREET: Plaza Dr

DATE: 09/12/24

LOCATION: Apache Junction

E-W STREET: Superstition Blvd

DAY: THURSDAY

PROJECT# 24-1477-001

	NORTHBOUND			SOUTHBOUND			EASTBOUND			WESTBOUND			
LANES:	NL 1	NT 0.5	NR 0.5	SL 1	ST 0.5	SR 0.5	EL 0	ET 2	ER 0	WL 0	WT 2	WR 0	TOTAL
6:00 AM													
6:15 AM													
6:30 AM													
6:45 AM													
7:00 AM	4	0	0	0	0	3	1	49	9	1	31	0	98
7:15 AM	3	1	1	1	2	2	4	35	11	5	36	0	101
7:30 AM	4	1	4	1	0	3	2	52	15	7	42	2	133
7:45 AM	1	0	1	2	1	2	2	58	12	2	40	0	121
8:00 AM	3	0	4	2	1	1	2	47	11	7	66	0	144
8:15 AM	6	1	1	1	2	1	4	32	10	6	54	2	120
8:30 AM	2	0	1	2	1	1	1	57	5	4	41	1	116
8:45 AM	5	1	8	2	0	1	3	56	11	6	38	0	131
9:00 AM													
9:15 AM													
9:30 AM													
9:45 AM													
10:00 AM													
10:15 AM													
10:30 AM													
10:45 AM													
11:00 AM													
11:15 AM													
11:30 AM													
11:45 AM													

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
Volumes	28	4	20	11	7	14	19	386	84	38	348	5	964
Approach %	53.85	7.69	38.46	34.38	21.88	43.75	3.89	78.94	17.18	9.72	89.00	1.28	
App/Depart	52	/	28	32	/	129	489	/	417	391	/	390	

AM Peak Hr Begins at: 730 AM

PEAK

Volumes	14	2	10	6	4	7	10	189	48	22	202	4	518
Approach %	53.85	7.69	38.46	35.29	23.53	41.18	4.05	76.52	19.43	9.65	88.60	1.75	

PEAK HR.

FACTOR:	0.722	0.850	0.858	0.781	0.899
---------	-------	-------	-------	-------	-------

CONTROL: 2-Way Stop (NB & SB)

COMMENT 1:

GPS: 33.422335, -111.550436

Intersection Turning Movement



FIELD DATA SERVICES OF ARIZONA, INC.
520.316.6745



veracity**traffic**group

N-S STREET: Plaza Dr

DATE: 09/12/24

LOCATION: Apache Junction

E-W STREET: Superstition Blvd

DAY: THURSDAY

PROJECT# 24-1477-001

	NORTHBOUND			SOUTHBOUND			EASTBOUND			WESTBOUND			
LANES:	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
	1	0.5	0.5	1	0.5	0.5	0	2	0	0	2	0	
1:00 PM													
1:15 PM													
1:30 PM													
1:45 PM													
2:00 PM													
2:15 PM													
2:30 PM													
2:45 PM													
3:00 PM													
3:15 PM													
3:30 PM													
3:45 PM													
4:00 PM	6	1	3	3	0	4	4	68	10	4	68	1	172
4:15 PM	5	2	5	0	0	3	1	60	2	6	58	1	143
4:30 PM	6	3	4	4	0	5	1	73	5	3	66	2	172
4:45 PM	4	4	5	0	1	1	1	67	2	2	49	4	140
5:00 PM	15	1	9	2	1	2	3	72	4	2	59	4	174
5:15 PM	6	0	8	1	1	5	4	67	4	3	44	2	145
5:30 PM	6	2	4	2	0	2	1	56	4	0	52	0	129
5:45 PM	1	0	2	0	0	1	5	48	7	1	49	5	119
6:00 PM													
6:15 PM													
6:30 PM													
6:45 PM													

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
Volumes	49	13	40	12	3	23	20	511	38	21	445	19	1194
Approach %	48.04	12.75	39.22	31.58	7.89	60.53	3.51	89.81	6.68	4.33	91.75	3.92	
App/Depart	102	/	52	38	/	62	569	/	563	485	/	517	

PM Peak Hr Begins at: 430 PM

PEAK

Volumes	31	8	26	7	3	13	9	279	15	10	218	12	631
Approach %	47.69	12.31	40.00	30.43	13.04	56.52	2.97	92.08	4.95	4.17	90.83	5.00	

PEAK HR.

FACTOR:	0.650	0.639	0.959	0.845	0.907
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CONTROL: 2-Way Stop (NB & SB)

COMMENT 1: 0

GPS: 33.422335, -111.550436



FIELD DATA SERVICES OF ARIZONA, INC.
520.316.6745



veracitytrafficgroup

Pedestrian & Bicycle Study

N-S STREET: Plaza Dr
E-W STREET: Superstition Blvd

Date: 09/12/24
Day: THURSDAY

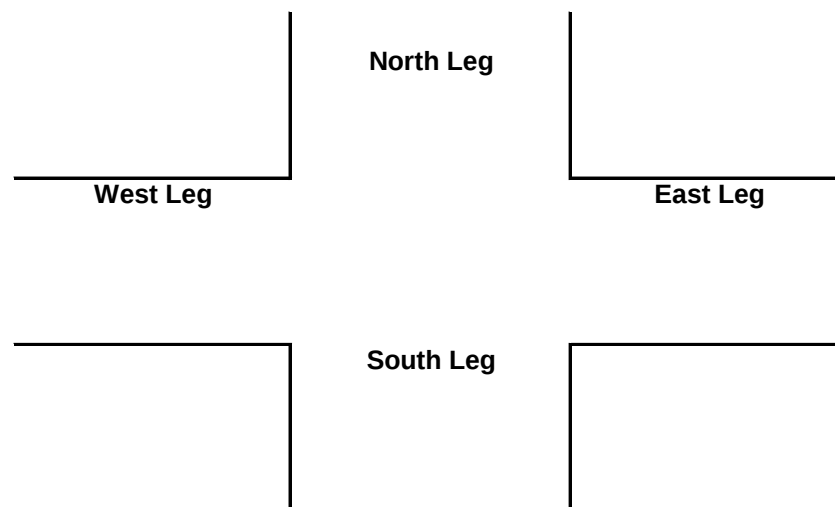
City: Apache Junction
Project #: 24-1477-001

	PEDESTRIANS			
	N-LEG	S-LEG	E-LEG	W-LEG
7:00 AM	0	0	0	0
7:15 AM	0	0	0	0
7:30 AM	0	0	0	0
7:45 AM	0	0	0	0
8:00 AM	0	0	0	0
8:15 AM	0	0	0	0
8:30 AM	0	0	0	0
8:45 AM	1	0	0	0
TOTAL	1	0	0	0

	BICYCLES			
	N-LEG	S-LEG	E-LEG	W-LEG
7:00 AM	0	0	0	0
7:15 AM	1	0	0	0
7:30 AM	0	0	0	0
7:45 AM	0	0	0	0
8:00 AM	0	0	0	0
8:15 AM	1	0	0	0
8:30 AM	0	0	0	0
8:45 AM	0	1	0	0
TOTAL	2	1	0	0

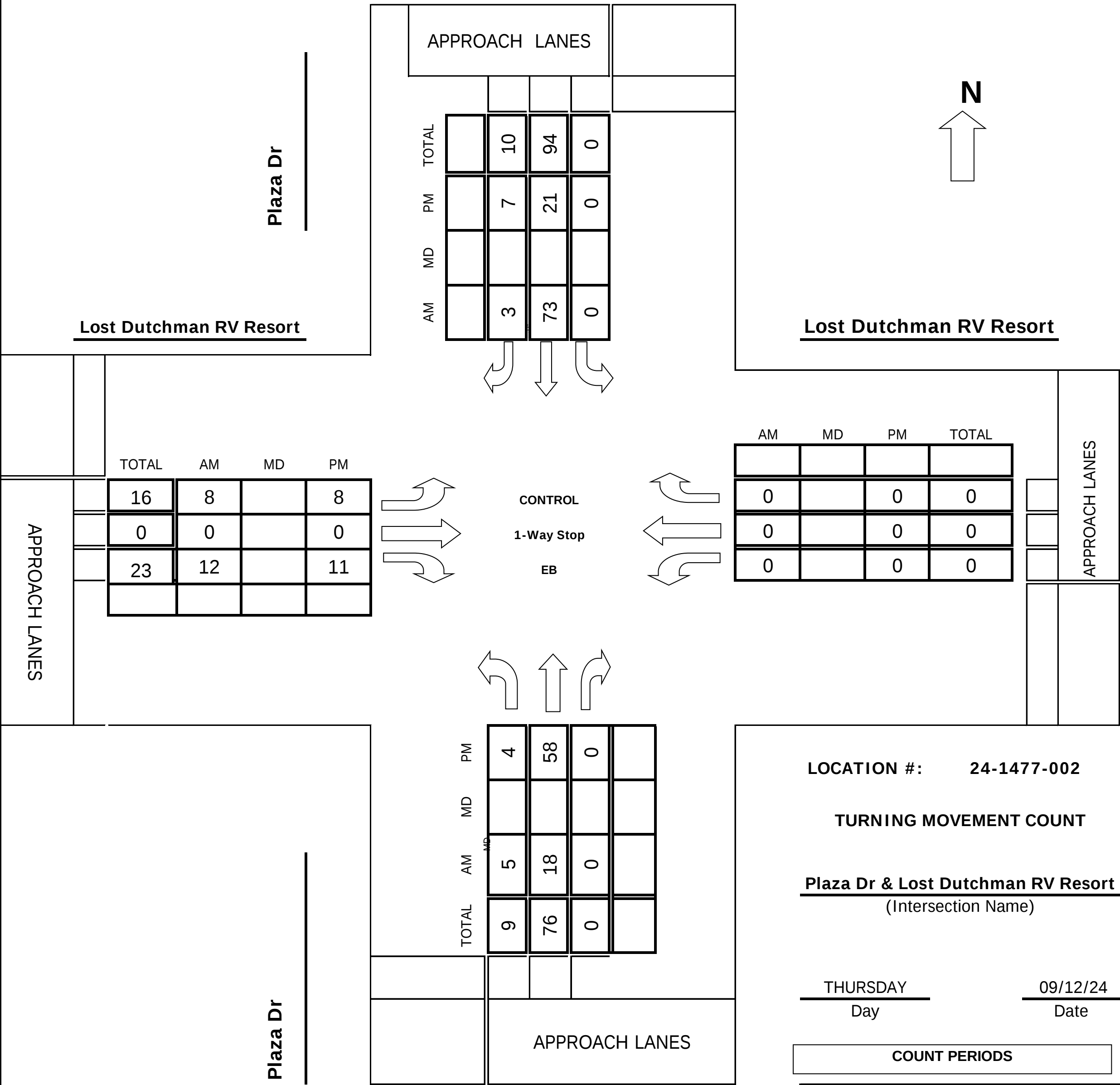
	PEDESTRIANS			
	N-LEG	S-LEG	E-LEG	W-LEG
4:00 PM	0	0	0	0
4:15 PM	0	0	0	0
4:30 PM	0	0	0	0
4:45 PM	0	0	0	0
5:00 PM	0	0	0	0
5:15 PM	0	0	0	0
5:30 PM	0	0	1	0
5:45 PM	0	0	0	0
TOTAL	0	0	1	0

	BICYCLES			
	N-LEG	S-LEG	E-LEG	W-LEG
4:00 PM	0	0	0	0
4:15 PM	0	0	0	0
4:30 PM	1	0	1	0
4:45 PM	0	0	0	0
5:00 PM	0	0	0	0
5:15 PM	0	0	0	0
5:30 PM	0	0	0	0
5:45 PM	1	0	0	0
TOTAL	2	0	1	0



Project #: 24-1477-002

TMC SUMMARY OF Plaza Dr & Lost Dutchman RV Resort



LOCATION #: 24-1477-002

TURNING MOVEMENT COUNT

Plaza Dr & Lost Dutchman RV Resort
(Intersection Name)

THURSDAY 09/12/24
Day Date

COUNT PERIODS			
AM	700AM	-	900AM
NOON		-	
PM	400PM	-	600PM

AM PEAK HOUR 730 AM

NOON PEAK HOUR

PM PEAK HOUR 430 PM

Intersection Turning Movement

Prepared by:



FIELD DATA SERVICES OF ARIZONA, INC.
520.316.6745



veracitytrafficgroup

N-S STREET: Plaza Dr

DATE: 09/12/24

LOCATION: Apache Junction

E-W STREET: Lost Dutchman RV Resort

DAY: THURSDAY

PROJECT# 24-1477-002

	NORTHBOUND			SOUTHBOUND			EASTBOUND			WESTBOUND			
LANES:	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
	0	1	0	0	1	0	1	1	1	0	0	0	
6:00 AM													
6:15 AM													
6:30 AM													
6:45 AM													
7:00 AM	1	2	0	0	8	1	1	0	3	0	0	0	16
7:15 AM	1	3	0	0	16	1	2	0	1	0	0	0	24
7:30 AM	2	4	0	0	22	0	4	0	3	0	0	0	35
7:45 AM	0	2	0	0	16	0	1	0	4	0	0	0	23
8:00 AM	3	5	0	0	18	1	2	0	3	0	0	0	32
8:15 AM	0	7	0	0	17	2	1	0	2	0	0	0	29
8:30 AM	1	3	0	0	10	1	1	0	1	0	0	0	17
8:45 AM	0	10	0	0	14	2	2	0	4	0	0	0	32
9:00 AM													
9:15 AM													
9:30 AM													
9:45 AM													
10:00 AM													
10:15 AM													
10:30 AM													
10:45 AM													
11:00 AM													
11:15 AM													
11:30 AM													
11:45 AM													

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
Volumes	8	36	0	0	121	8	14	0	21	0	0	0	208
Approach %	18.18	81.82	0.00	0.00	93.80	6.20	40.00	0.00	60.00	####	####	####	
App/Depart	44	/	50	129	/	142	35	/	0	0	/	16	

AM Peak Hr Begins at: 730 AM

PEAK

Volumes	5	18	0	0	73	3	8	0	12	0	0	0	119
Approach %	21.74	78.26	0.00	0.00	96.05	3.95	40.00	0.00	60.00	####	####	####	

PEAK HR.

FACTOR:	0.719	0.864	0.714	0.000	0.850
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CONTROL: 1-Way Stop (EB)

COMMENT 1:

GPS: 33.418663, -111.550538

Intersection Turning Movement



FIELD DATA SERVICES OF ARIZONA, INC.
520.316.6745



veracity**traffic**group

N-S STREET: Plaza Dr

DATE: 09/12/24

LOCATION: Apache Junction

0

E-W STREET: Lost Dutchman RV Resort

DAY: THURSDAY

PROJECT# 24-1477-002

	NORTHBOUND			SOUTHBOUND			EASTBOUND			WESTBOUND			
LANES:	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
	0	1	0	0	1	0	1	1	1	0	0	0	
1:00 PM													
1:15 PM													
1:30 PM													
1:45 PM													
2:00 PM													
2:15 PM													
2:30 PM													
2:45 PM													
3:00 PM													
3:15 PM													
3:30 PM													
3:45 PM													
4:00 PM	3	10	0	0	10	1	1	0	1	0	0	0	26
4:15 PM	2	10	0	0	6	2	2	0	2	0	0	0	24
4:30 PM	1	9	0	0	8	1	3	0	5	0	0	0	27
4:45 PM	0	12	0	0	3	3	2	0	2	0	0	0	22
5:00 PM	1	23	0	0	5	2	2	0	3	0	0	0	36
5:15 PM	2	14	0	0	5	1	1	0	1	0	0	0	24
5:30 PM	1	9	0	0	3	1	4	0	4	0	0	0	22
5:45 PM	3	3	0	0	4	2	2	0	1	0	0	0	15
6:00 PM													
6:15 PM													
6:30 PM													
6:45 PM													

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
Volumes	13	90	0	0	44	13	17	0	19	0	0	0	196
Approach %	12.62	87.38	0.00	0.00	77.19	22.81	47.22	0.00	52.78	####	####	####	
App/Depart	103	/	107	57	/	63	36	/	0	0	/	26	

PM Peak Hr Begins at: 430 PM

PEAK

Volumes	4	58	0	0	21	7	8	0	11	0	0	0	109
Approach %	6.45	93.55	0.00	0.00	75.00	25.00	42.11	0.00	57.89	####	####	####	

PEAK HR.

FACTOR:	0.646	0.778	0.594	0.000	0.757
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CONTROL: 1-Way Stop (EB)

COMMENT 1: 0

GPS: 33.418663, -111.550538



FIELD DATA SERVICES OF ARIZONA, INC.
520.316.6745



veracitytrafficgroup

Pedestrian & Bicycle Study

N-S STREET: Plaza Dr

E-W STREET: Lost Dutchman RV Resort

Date: 09/12/24

Day: THURSDAY

City: Apache Junction

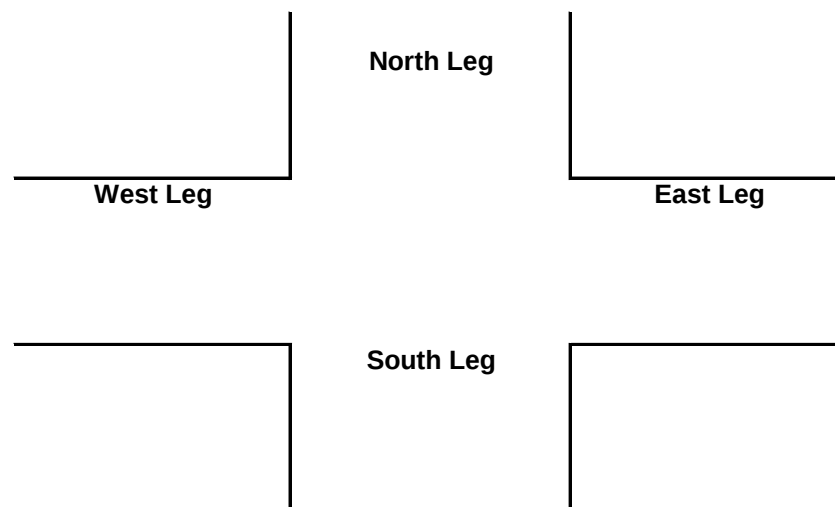
Project #: 24-1477-002

	PEDESTRIANS			
	N-LEG	S-LEG	E-LEG	W-LEG
7:00 AM	0	0	0	0
7:15 AM	0	0	0	0
7:30 AM	0	0	0	0
7:45 AM	0	0	0	0
8:00 AM	0	0	0	0
8:15 AM	0	0	0	0
8:30 AM	0	0	0	0
8:45 AM	1	0	0	0
TOTAL	1	0	0	0

	BICYCLES			
	N-LEG	S-LEG	E-LEG	W-LEG
7:00 AM	0	0	0	0
7:15 AM	0	0	0	0
7:30 AM	0	0	0	0
7:45 AM	0	0	0	0
8:00 AM	0	0	0	0
8:15 AM	0	0	0	0
8:30 AM	0	0	0	0
8:45 AM	0	0	0	0
TOTAL	0	0	0	0

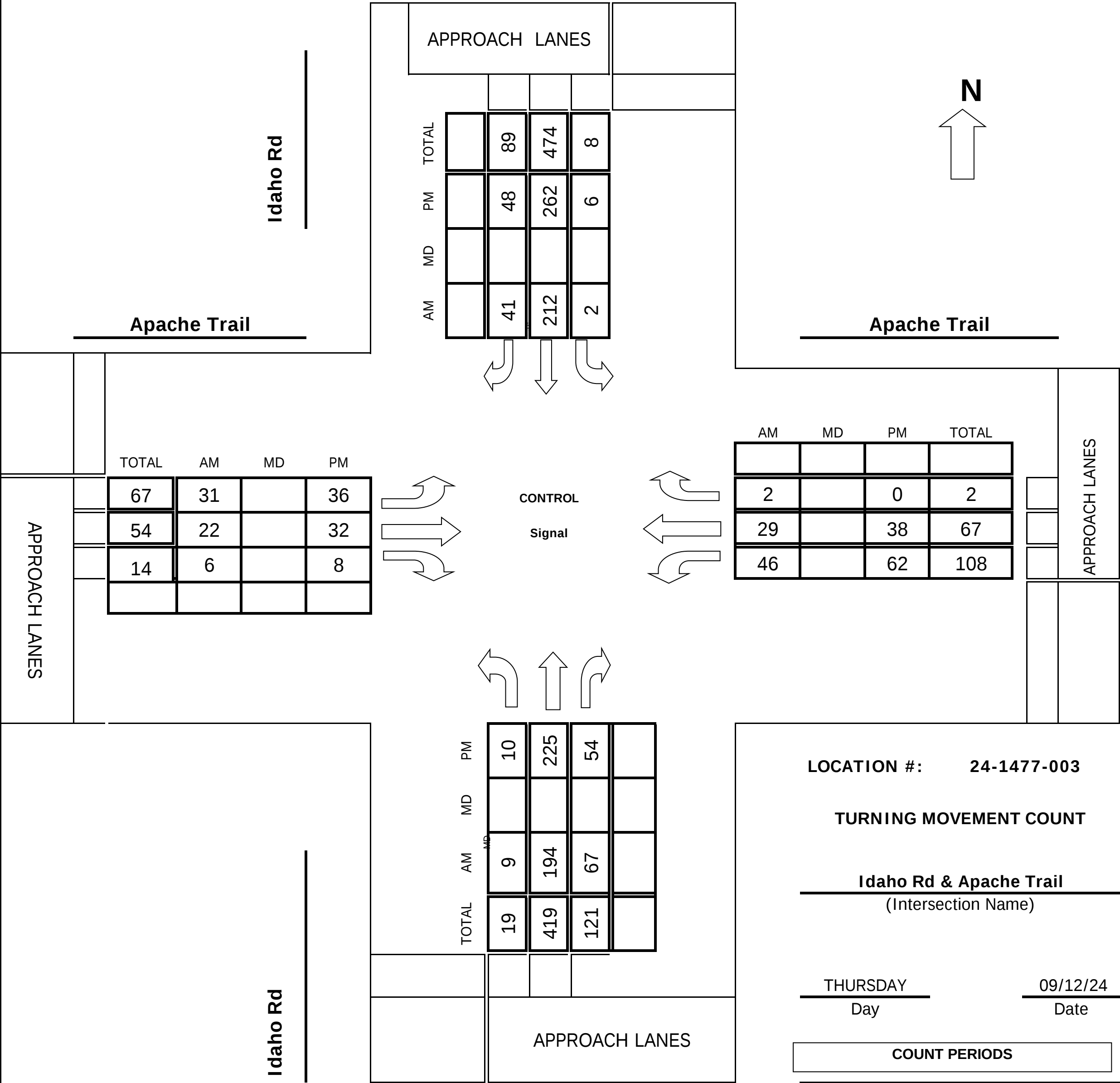
	PEDESTRIANS			
	N-LEG	S-LEG	E-LEG	W-LEG
4:00 PM	0	0	0	0
4:15 PM	0	0	0	0
4:30 PM	0	0	0	0
4:45 PM	0	0	0	0
5:00 PM	0	0	0	0
5:15 PM	0	0	0	0
5:30 PM	0	0	0	0
5:45 PM	0	0	0	0
TOTAL	0	0	0	0

	BICYCLES			
	N-LEG	S-LEG	E-LEG	W-LEG
4:00 PM	0	0	0	0
4:15 PM	0	0	0	0
4:30 PM	0	0	0	0
4:45 PM	0	0	0	0
5:00 PM	0	0	0	0
5:15 PM	0	0	0	0
5:30 PM	0	0	0	0
5:45 PM	0	0	0	0
TOTAL	0	0	0	0



Project #: 24-1477-003

TMC SUMMARY OF Idaho Rd & Apache Trail



AM PEAK HOUR 800 AM

NOON PEAK HOUR

PM PEAK HOUR 430 PM

Intersection Turning Movement

Prepared by:



FIELD DATA SERVICES OF ARIZONA, INC.
520.316.6745



veracitytrafficgroup

N-S STREET: Idaho Rd

DATE: 09/12/24

LOCATION: Apache Junction

E-W STREET: Apache Trail

DAY: THURSDAY

PROJECT# 24-1477-003

	NORTHBOUND			SOUTHBOUND			EASTBOUND			WESTBOUND			
LANES:	NL 1	NT 1.5	NR 1.5	SL 1	ST 2	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL
6:00 AM													
6:15 AM													
6:30 AM													
6:45 AM													
7:00 AM	1	41	16	1	24	5	2	2	0	16	3	1	112
7:15 AM	2	42	13	1	43	6	2	2	4	13	8	2	138
7:30 AM	4	46	19	0	30	9	4	3	2	9	6	1	133
7:45 AM	1	41	18	3	58	10	2	3	1	19	9	0	165
8:00 AM	1	48	16	0	42	11	9	2	2	11	5	1	148
8:15 AM	2	50	18	1	52	10	6	6	3	9	5	0	162
8:30 AM	2	51	22	1	61	5	6	5	1	15	9	1	179
8:45 AM	4	45	11	0	57	15	10	9	0	11	10	0	172
9:00 AM													
9:15 AM													
9:30 AM													
9:45 AM													
10:00 AM													
10:15 AM													
10:30 AM													
10:45 AM													
11:00 AM													
11:15 AM													
11:30 AM													
11:45 AM													

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
Volumes	17	364	133	7	367	71	41	32	13	103	55	6	1209
Approach %	3.31	70.82	25.88	1.57	82.47	15.96	47.67	37.21	15.12	62.80	33.54	3.66	
App/Depart	514	/	411	445	/	483	86	/	172	164	/	143	

AM Peak Hr Begins at: 800 AM

PEAK

Volumes	9	194	67	2	212	41	31	22	6	46	29	2	661
Approach %	3.33	71.85	24.81	0.78	83.14	16.08	52.54	37.29	10.17	59.74	37.66	2.60	

PEAK HR.

FACTOR:	0.900	0.885	0.776	0.770	0.923
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CONTROL: Signal

COMMENT 1:

GPS: 33.419040, -111.546090

Intersection Turning Movement



FIELD DATA SERVICES OF ARIZONA, INC.
520.316.6745



veracity**traffic**group

N-S STREET: [Idaho Rd](#)

DATE: [09/12/24](#)

LOCATION: [Apache Junction](#)

E-W STREET: [Apache Trail](#)

DAY: [THURSDAY](#)

PROJECT# [24-1477-003](#)

	NORTHBOUND			SOUTHBOUND			EASTBOUND			WESTBOUND			
	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
LANES:	1	1.5	1.5	1	2	0	1	2	0	1	2	0	
1:00 PM													
1:15 PM													
1:30 PM													
1:45 PM													
2:00 PM													
2:15 PM													
2:30 PM													
2:45 PM													
3:00 PM													
3:15 PM													
3:30 PM													
3:45 PM													
4:00 PM	2	75	15	2	67	13	9	7	2	15	5	0	212
4:15 PM	3	46	10	2	70	13	7	9	2	12	4	0	178
4:30 PM	5	49	15	1	66	9	11	5	4	14	15	0	194
4:45 PM	2	56	16	2	68	12	7	10	2	12	10	0	197
5:00 PM	2	59	10	3	57	14	8	8	1	21	3	0	186
5:15 PM	1	61	13	0	71	13	10	9	1	15	10	0	204
5:30 PM	5	48	11	0	50	10	8	6	2	13	7	0	160
5:45 PM	0	48	12	1	45	12	11	11	1	9	3	0	153
6:00 PM													
6:15 PM													
6:30 PM													
6:45 PM													

TOTAL	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
Volumes	20	442	102	11	494	96	71	65	15	111	57	0	1484
Approach %	3.55	78.37	18.09	1.83	82.20	15.97	47.02	43.05	9.93	66.07	33.93	0.00	
App/Depart	564	/	513	601	/	620	151	/	178	168	/	173	

PM Peak Hr Begins at: 430 PM

PEAK

Volumes	10	225	54	6	262	48	36	32	8	62	38	0	781
Approach %	3.46	77.85	18.69	1.90	82.91	15.19	47.37	42.11	10.53	62.00	38.00	0.00	

PEAK HR.

FACTOR:	0.963	0.940	0.950	0.862	0.957
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CONTROL: [Signal](#)

COMMENT 1: [0](#)

GPS: [33.419040, -111.546090](#)



FIELD DATA SERVICES OF ARIZONA, INC.
520.316.6745



veracitytrafficgroup

Pedestrian & Bicycle Study

N-S STREET: Idaho Rd
E-W STREET: Apache Trail

Date: 09/12/24
Day: THURSDAY

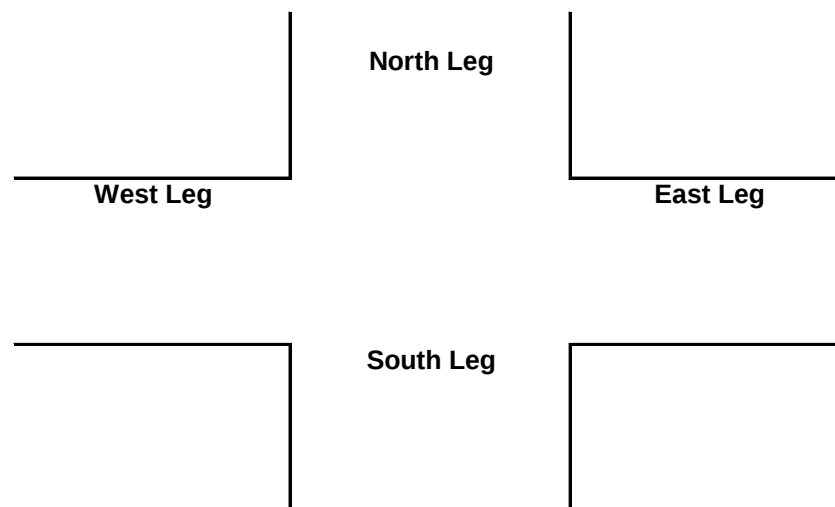
City: Apache Junction
Project #: 24-1477-003

	PEDESTRIANS			
	N-LEG	S-LEG	E-LEG	W-LEG
7:00 AM	0	0	0	0
7:15 AM	0	0	0	1
7:30 AM	0	0	0	0
7:45 AM	2	0	4	0
8:00 AM	0	0	0	0
8:15 AM	0	0	1	0
8:30 AM	0	0	0	0
8:45 AM	1	0	1	0
TOTAL	3	0	6	1

	BICYCLES			
	N-LEG	S-LEG	E-LEG	W-LEG
7:00 AM	0	0	0	0
7:15 AM	0	0	1	0
7:30 AM	0	0	0	0
7:45 AM	0	0	0	0
8:00 AM	0	0	1	0
8:15 AM	0	0	0	0
8:30 AM	0	0	0	0
8:45 AM	0	0	0	0
TOTAL	0	0	2	0

	PEDESTRIANS			
	N-LEG	S-LEG	E-LEG	W-LEG
4:00 PM	0	0	0	0
4:15 PM	0	0	2	0
4:30 PM	0	0	1	0
4:45 PM	0	0	0	0
5:00 PM	0	0	0	0
5:15 PM	0	0	0	0
5:30 PM	0	0	0	1
5:45 PM	0	0	0	1
TOTAL	0	0	3	2

	BICYCLES			
	N-LEG	S-LEG	E-LEG	W-LEG
4:00 PM	0	0	0	0
4:15 PM	0	0	0	0
4:30 PM	0	0	0	0
4:45 PM	0	0	0	0
5:00 PM	0	0	0	0
5:15 PM	0	0	1	0
5:30 PM	1	0	0	0
5:45 PM	0	0	0	0
TOTAL	1	0	1	0



Prepared by: Field Data Services of Arizona/Veracity Traffic Group (520) 316-6745

Volumes for: Thursday, September 12, 2024

City: Apache Junction

Project #: 24-1477-005

Location: Plaza Dr south of Superstition Blvd

AM Period	NB		SB		EB	WB	PM Period	NB		SB		EB	WB	
00:00	1		0				12:00	12		15				
00:15	0		0				12:15	13		27				
00:30	0		0				12:30	20		19				
00:45	0	1	0	0		1	12:45	17	62	23	84		146	
01:00	0		0				13:00	15		13				
01:15	0		0				13:15	11		15				
01:30	0		0				13:30	15		12				
01:45	0	0	0	0			13:45	13	54	22	62		116	
02:00	0		1				14:00	10		18				
02:15	0		0				14:15	11		12				
02:30	0		1				14:30	21		14				
02:45	0	0	2	4		4	14:45	12	54	16	60		114	
03:00	1		1				15:00	16		11				
03:15	0		0				15:15	17		14				
03:30	0		0				15:30	13		9				
03:45	0	1	0	1		2	15:45	20	66	9	43		109	
04:00	0		0				16:00	10		14				
04:15	1		0				16:15	12		8				
04:30	2		2				16:30	13		8				
04:45	1	4	2	4		8	16:45	13	48	5	35		83	
05:00	0		3				17:00	25		7				
05:15	1		1				17:15	14		8				
05:30	1		1				17:30	12		4				
05:45	0	2	7	12		14	17:45	3	54	8	27		81	
06:00	3		7				18:00	13		5				
06:15	3		6				18:15	2		6				
06:30	0		9				18:30	6		3				
06:45	2	8	18	40		48	18:45	5	26	1	15		41	
07:00	4		10				19:00	5		3				
07:15	5		18				19:15	6		2				
07:30	9		22				19:30	5		8				
07:45	2	20	15	65		85	19:45	6	22	5	18		40	
08:00	7		19				20:00	3		1				
08:15	8		18				20:15	3		1				
08:30	3		10				20:30	3		0				
08:45	14	32	17	64		96	20:45	2	11	1	3		14	
09:00	30		12				21:00	2		5				
09:15	33		12				21:15	0		2				
09:30	29		10				21:30	1		3				
09:45	10	102	10	44		146	21:45	0	3	0	10		13	
10:00	11		13				22:00	1		1				
10:15	9		12				22:15	1		0				
10:30	12		22				22:30	1		0				
10:45	15	47	21	68		115	22:45	0	3	0	1		4	
11:00	15		10				23:00	0		0				
11:15	19		23				23:15	0		0				
11:30	16		12				23:30	0		0				
11:45	15	65	24	69		134	23:45	1	1	0	0		1	
Total Vol.	282		371			653		404		358			762	
GPS Coordinates:		33.421540, -111.550436						Daily Totals						
								NB	SB	EB	WB	Combined		
								686	729			1415		
		AM						PM						
Split %	43.2%		56.8%			46.1 %		53.0%	47.0%			53.9 %		
Peak Hour	08:45		11:45			08:45		14:30	12:00			12:15		
Volume	106		85			157		66	84			147		
P.H.F.	0.80		0.79			0.87		0.79	0.78			0.92		

Prepared by: Field Data Services of Arizona/Veracity Traffic Group (520) 316-6745

Volumes for: Thursday, September 12, 2024

City: Apache Junction

Project #: 24-1477-006

Location: Idaho Rd north of Apache Trail

AM Period	NB		SB		EB	WB	PM Period	NB		SB		EB	WB
00:00	5		3				12:00	77		98			
00:15	3		3				12:15	80		87			
00:30	5		2				12:30	77		87			
00:45	7	20	0	8		28	12:45	93	327	90	362		689
01:00	2		3				13:00	102		91			
01:15	3		3				13:15	91		57			
01:30	3		2				13:30	77		90			
01:45	3	11	0	8		19	13:45	93	363	82	320		683
02:00	0		1				14:00	73		111			
02:15	1		4				14:15	83		88			
02:30	1		3				14:30	73		74			
02:45	1	3	2	10		13	14:45	72	301	73	346		647
03:00	2		6				15:00	80		84			
03:15	3		2				15:15	82		93			
03:30	4		7				15:30	70		91			
03:45	6	15	7	22		37	15:45	66	298	109	377		675
04:00	8		11				16:00	84		82			
04:15	9		6				16:15	53		85			
04:30	11		13				16:30	60		76			
04:45	15	43	17	47		90	16:45	63	260	82	325		585
05:00	15		18				17:00	67		74			
05:15	23		16				17:15	71		84			
05:30	19		21				17:30	56		60			
05:45	21	78	19	74		152	17:45	59	253	58	276		529
06:00	20		43				18:00	54		73			
06:15	38		34				18:15	41		52			
06:30	43		41				18:30	53		36			
06:45	53	154	36	154		308	18:45	40	188	46	207		395
07:00	44		30				19:00	45		43			
07:15	46		50				19:15	43		30			
07:30	51		39				19:30	41		44			
07:45	43	184	71	190		374	19:45	42	171	35	152		323
08:00	58		53				20:00	30		43			
08:15	56		63				20:15	32		43			
08:30	58		67				20:30	24		32			
08:45	55	227	72	255		482	20:45	24	110	20	138		248
09:00	75		70				21:00	17		30			
09:15	70		84				21:15	11		26			
09:30	58		102				21:30	16		12			
09:45	93	296	78	334		630	21:45	13	57	16	84		141
10:00	71		80				22:00	16		13			
10:15	76		79				22:15	9		9			
10:30	75		88				22:30	11		13			
10:45	75	297	84	331		628	22:45	10	46	6	41		87
11:00	81		69				23:00	12		8			
11:15	58		99				23:15	9		16			
11:30	89		108				23:30	4		6			
11:45	82	310	91	367		677	23:45	8	33	7	37		70

Total Vol.	1638	1800	3438	2407	2665	5072		
GPS Coordinates:	33.419939, -111.546105			Daily Totals				
				NB	SB	EB	WB	Combined
				4045	4465			8510

AM				PM			
Split %	47.6%	52.4%	40.4%	47.5%	52.5%		59.6%
Peak Hour	11:30	11:15	11:30	12:30	15:00		12:15
Volume	328	396	712	363	377		707
P.H.F.	0.92	0.92	0.90	0.89	0.86		0.92

Prepared by: Field Data Services of Arizona/Veracity Traffic Group (520) 316-6745

Volumes for: Thursday, September 12, 2024

City: Apache Junction

Project #: 24-1477-007

Location: Apache Trail south west of Idaho Rd

AM Period	NB	SB	EB	WB	PM Period	NB	SB	EB	WB				
00:00			1	2	12:00			25	37				
00:15			2	1	12:15			14	28				
00:30			0	0	12:30			27	36				
00:45			1	4	0	3	7	12:45	30	96	26	127	223
01:00			0	3	13:00			32	23				
01:15			0	0	13:15			27	17				
01:30			1	1	13:30			24	27				
01:45			0	1	0	4	5	13:45	22	105	30	97	202
02:00			0	4	14:00			21	34				
02:15			0	1	14:15			19	21				
02:30			0	0	14:30			25	22				
02:45			0	0	0	5	5	14:45	31	96	20	97	193
03:00			0	1	15:00			15	20				
03:15			0	0	15:15			20	12				
03:30			1	0	15:30			19	29				
03:45			1	2	0	1	3	15:45	20	74	32	93	167
04:00			1	0	16:00			18	20				
04:15			0	0	16:15			18	20				
04:30			1	3	16:30			20	29				
04:45			3	5	5	8	13	16:45	19	75	24	93	168
05:00			2	1	17:00			17	19				
05:15			2	1	17:15			20	24				
05:30			1	0	17:30			16	22				
05:45			5	10	6	8	18	17:45	23	76	15	80	156
06:00			3	9	18:00			16	12				
06:15			4	10	18:15			17	16				
06:30			8	6	18:30			19	20				
06:45			7	22	11	36	58	18:45	21	73	20	68	141
07:00			4	9	19:00			17	17				
07:15			8	16	19:15			15	10				
07:30			9	19	19:30			13	14				
07:45			6	27	20	64	91	19:45	16	61	11	52	113
08:00			13	17	20:00			10	9				
08:15			15	17	20:15			9	11				
08:30			12	16	20:30			9	9				
08:45			19	59	29	79	138	20:45	8	36	6	35	71
09:00			16	23	21:00			4	5				
09:15			8	30	21:15			7	4				
09:30			21	35	21:30			5	1				
09:45			22	67	33	121	188	21:45	3	19	4	14	33
10:00			22	30	22:00			4	4				
10:15			23	27	22:15			4	1				
10:30			19	37	22:30			4	6				
10:45			20	84	35	129	213	22:45	0	12	1	12	24
11:00			23	21	23:00			3	0				
11:15			18	44	23:15			2	2				
11:30			19	31	23:30			0	1				
11:45			24	84	26	122	206	23:45	1	6	2	5	11

Total Vol.	365	580	945	729	773	1502
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GPS Coordinates: 33.418036, -111.547100

				Daily Totals		
				NB	SB	Combined
				1094	1353	2447
AM				PM		
Split %	38.6%	61.4%	38.6 %	48.5%	51.5%	61.4 %
Peak Hour	11:45	11:15	11:15	12:30	12:00	12:00
Volume	90	138	224	116	127	223
P.H.F.	0.83	0.78	0.90	0.91	0.86	0.88

APPENDIX C: Seasonal Factor Calculation

DAILY TRAFFIC	2016	2015	2013	2012	Source: ADOT & MAG Traffic Count Data System (TCDS)	
Superstition Blvd, west of Idaho Rd (EB Approach)	(Winter) 6,312	(Summer) 4,004	(Winter) 6,999	(Summer) 4,192		
% Difference		36.57%	42.79%	40.11%		
% Difference (Average)			41.45%			
DAILY TRAFFIC	2021	2020	2019	2017	2016	Source: ADOT & MAG Traffic Count Data System (TCDS)
Idaho Rd, south of Apache Trail (NB/SB Approaches)	(Spring) 9,204	(Summer) 7,367	(Winter) 9,327	(Spring) 10,447	(Winter) 7,866	
% Difference		19.96%	21.01%	10.72%	24.71%	
% Difference (Average)			19.10%			
DAILY TRAFFIC	2022	2020	2019	2018	2014	Source: ADOT & MAG Traffic Count Data System (TCDS)
Apache Trail, east of Idaho Rd (EB/WB Approaches)	(Spring) 3,540	(Spring) 3,618	(Summer) 2,372	(Winter) 3,077	(Winter) 3,598	
% Difference		-	34.44%	22.91%	-	
% Difference (Average)			28.68%			
DAILY TRAFFIC	2025	2024	Source: City of Apache Junction & Current Count Data			
Plaza Dr, south of Superstition Blvd (NB/SB Approaches)	(Spring) 2,255	(Summer) 1,415				
% Difference		37.25%				
TOTAL AVERAGE SEASONAL DIFFERENCE		31.62%	[Used 35%]			

APPENDIX D: Existing Level-of-Service Analysis

1: Site Access A/Plaza Dr & Superstition Blvd
2024 EXISTING CONDITIONS - AM Peak Hour

HCM 6th TWSC

Intersection												
Int Delay, s/veh	1.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	14	255	65	30	273	5	19	3	14	8	5	9
Future Vol, veh/h	14	255	65	30	273	5	19	3	14	8	5	9
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	0	-	-	0	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	15	277	71	33	297	5	21	3	15	9	5	10
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	302	0	0	348	0	0	560	711	174	536	744	151
Stage 1	-	-	-	-	-	-	343	343	-	366	366	-
Stage 2	-	-	-	-	-	-	217	368	-	170	378	-
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32
Pot Cap-1 Maneuver	1256	-	-	1208	-	-	411	357	839	428	341	868
Stage 1	-	-	-	-	-	-	646	636	-	626	621	-
Stage 2	-	-	-	-	-	-	765	620	-	815	614	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1256	-	-	1208	-	-	389	343	839	405	328	868
Mov Cap-2 Maneuver	-	-	-	-	-	-	389	343	-	405	328	-
Stage 1	-	-	-	-	-	-	638	628	-	618	604	-
Stage 2	-	-	-	-	-	-	729	603	-	787	607	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0.8			12.8			12.6		
HCM LOS							B			B		
Minor Lane/Major Mvmt	NBLn1		NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2	
Capacity (veh/h)	389		668	1256	-	-	1208	-	-	405	547	
HCM Lane V/C Ratio	0.053		0.028	0.012	-	-	0.027	-	-	0.021	0.028	
HCM Control Delay (s)	14.8		10.5	7.9	-	-	8.1	-	-	14.1	11.8	
HCM Lane LOS	B		B	A	-	-	A	-	-	B	B	
HCM 95th %tile Q(veh)	0.2		0.1	0	-	-	0.1	-	-	0.1	0.1	

2: Plaza Dr & Existing Driveway
2024 EXISTING CONDITIONS - AM Peak Hour

HCM 6th TWSC

Intersection						
Int Delay, s/veh	1.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	11	16	7	24	99	4
Future Vol, veh/h	11	16	7	24	99	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	12	17	8	26	108	4

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	152	110	112	0	-	0
Stage 1	110	-	-	-	-	-
Stage 2	42	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	840	943	1478	-	-	-
Stage 1	915	-	-	-	-	-
Stage 2	980	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	836	943	1478	-	-	-
Mov Cap-2 Maneuver	836	-	-	-	-	-
Stage 1	910	-	-	-	-	-
Stage 2	980	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	9.2	1.7	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1478	-	896	-	-
HCM Lane V/C Ratio	0.005	-	0.033	-	-
HCM Control Delay (s)	7.4	0	9.2	-	-
HCM Lane LOS	A	A	A	-	-
HCM 95th %tile Q(veh)	0	-	0.1	-	-

3: Idaho Rd & Apache Tr

2024 EXISTING CONDITIONS - AM Peak Hour

HCM 6th Signalized Intersection Summary

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	42	30	8	62	39	3	12	262	90	3	286	55
Future Volume (veh/h)	42	30	8	62	39	3	12	262	90	3	286	55
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	46	33	9	67	42	3	13	285	98	3	311	60
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	537	450	118	538	544	38	535	1067	452	558	849	162
Arrive On Green	0.16	0.16	0.16	0.16	0.16	0.16	0.29	0.29	0.29	0.29	0.29	0.29
Sat Flow, veh/h	1361	2787	729	1365	3367	238	1011	3741	1585	1000	2978	567
Grp Volume(v), veh/h	46	21	21	67	22	23	13	285	98	3	184	187
Grp Sat Flow(s),veh/h/ln	1361	1777	1739	1365	1777	1828	1011	1870	1585	1000	1777	1768
Q Serve(g_s), s	0.6	0.2	0.2	1.0	0.2	0.2	0.2	1.3	1.0	0.1	1.8	1.8
Cycle Q Clear(g_c), s	0.9	0.2	0.2	1.2	0.2	0.2	2.1	1.3	1.0	1.3	1.8	1.8
Prop In Lane	1.00		0.42	1.00		0.13	1.00		1.00	1.00		0.32
Lane Grp Cap(c), veh/h	537	287	281	538	287	295	535	1067	452	558	507	504
V/C Ratio(X)	0.09	0.07	0.08	0.12	0.08	0.08	0.02	0.27	0.22	0.01	0.36	0.37
Avail Cap(c_a), veh/h	2514	2868	2807	2521	2868	2950	2252	7418	3143	2257	3524	3507
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	8.1	7.7	7.7	8.2	7.7	7.7	7.0	6.0	5.9	6.5	6.2	6.2
Incr Delay (d2), s/veh	0.1	0.1	0.1	0.1	0.1	0.1	0.0	0.1	0.2	0.0	0.4	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.1	0.0	0.1	0.2	0.1	0.1	0.0	0.1	0.1	0.0	0.2	0.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	8.2	7.8	7.8	8.3	7.8	7.8	7.0	6.1	6.1	6.5	6.6	6.7
LnGrp LOS	A	A	A	A	A	A	A	A	A	A	A	A
Approach Vol, veh/h	88				112				396			
Approach Delay, s/veh	8.0				8.1				6.2			
Approach LOS	A				A				A			
Timer - Assigned Phs	2			4			6			8		
Phs Duration (G+Y+Rc), s	12.2			9.5			12.2			9.5		
Change Period (Y+Rc), s	6.0			6.0			6.0			6.0		
Max Green Setting (Gmax), s	43.0			35.0			43.0			35.0		
Max Q Clear Time (g_c+I1), s	4.1			2.9			3.8			3.2		
Green Ext Time (p_c), s	2.1			0.3			2.1			0.4		

Intersection Summary

HCM 6th Ctrl Delay	6.7
HCM 6th LOS	A

Notes

User approved volume balancing among the lanes for turning movement.

4: Apache Tr/Phelps Dr & Old West Hwy

2024 EXISTING CONDITIONS - AM Peak Hour

HCM 6th Signalized Intersection Summary



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↰ ↱ ↑↑↓			↰ ↱ ↑↑↓			↰ ↱ ↑↑↓			↰ ↱ ↑↑↓		
Traffic Volume (veh/h)	55	363	41	19	423	28	51	45	23	28	27	69
Future Volume (veh/h)	55	363	41	19	423	28	51	45	23	28	27	69
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	60	395	45	21	460	30	43	66	25	30	64	52
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	158	825	92	41	757	49	659	1109	398	674	774	656
Arrive On Green	0.05	0.18	0.18	0.02	0.15	0.15	0.04	0.42	0.42	0.03	0.41	0.41
Sat Flow, veh/h	3456	4659	521	1781	4901	317	1781	2628	943	1781	1870	1585
Grp Volume(v), veh/h	60	287	153	21	318	172	43	46	45	30	64	52
Grp Sat Flow(s),veh/h/ln	1728	1702	1777	1781	1702	1813	1781	1870	1701	1781	1870	1585
Q Serve(g_s), s	1.4	6.1	6.3	0.9	7.0	7.1	1.1	1.2	1.3	0.8	1.7	1.6
Cycle Q Clear(g_c), s	1.4	6.1	6.3	0.9	7.0	7.1	1.1	1.2	1.3	0.8	1.7	1.6
Prop In Lane	1.00		0.29	1.00		0.17	1.00		0.55	1.00		1.00
Lane Grp Cap(c), veh/h	158	603	315	41	526	280	659	789	717	674	774	656
V/C Ratio(X)	0.38	0.48	0.49	0.51	0.61	0.61	0.07	0.06	0.06	0.04	0.08	0.08
Avail Cap(c_a), veh/h	472	1605	838	243	1605	855	790	789	717	818	789	669
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	37.3	29.8	29.9	38.9	31.8	31.8	12.4	13.8	13.8	12.7	14.3	14.3
Incr Delay (d2), s/veh	1.5	0.6	1.2	9.2	1.1	2.2	0.0	0.1	0.2	0.0	0.0	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	2.5	2.7	0.5	2.9	3.2	0.4	0.5	0.5	0.3	0.7	0.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	38.8	30.4	31.0	48.1	32.9	34.0	12.4	13.9	14.0	12.7	14.4	14.4
LnGrp LOS	D	C	C	D	C	C	B	B	B	B	B	B
Approach Vol, veh/h	500			511			134			146		
Approach Delay, s/veh	31.6			33.9			13.5			14.0		
Approach LOS	C			C			B			B		
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	9.4	41.0	8.9	21.3	10.1	40.4	10.7	19.5				
Change Period (Y+Rc), s	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0				
Max Green Setting (Gmax), s	9.0	34.0	11.0	38.0	9.0	34.0	11.0	38.0				
Max Q Clear Time (g_c+I), s	12.8	3.3	2.9	8.3	3.1	3.7	3.4	9.1				
Green Ext Time (p_c), s	0.0	0.5	0.0	3.0	0.0	0.5	0.1	3.3				

Intersection Summary

HCM 6th Ctrl Delay 28.6

HCM 6th LOS C

Notes

User approved volume balancing among the lanes for turning movement.

1: Site Access A/Plaza Dr & Superstition Blvd
2024 EXISTING CONDITIONS - PM Peak Hour

HCM 6th TWSC

Intersection												
Int Delay, s/veh	2.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	12	377	20	14	294	16	42	11	35	9	4	18
Future Vol, veh/h	12	377	20	14	294	16	42	11	35	9	4	18
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	0	-	-	0	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	13	410	22	15	320	17	46	12	38	10	4	20
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	337	0	0	432	0	0	639	814	216	596	817	169
Stage 1	-	-	-	-	-	-	447	447	-	359	359	-
Stage 2	-	-	-	-	-	-	192	367	-	237	458	-
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32
Pot Cap-1 Maneuver	1219	-	-	1124	-	-	361	311	789	387	309	845
Stage 1	-	-	-	-	-	-	560	572	-	632	626	-
Stage 2	-	-	-	-	-	-	791	621	-	745	565	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1219	-	-	1124	-	-	343	304	789	351	302	845
Mov Cap-2 Maneuver	-	-	-	-	-	-	343	304	-	351	302	-
Stage 1	-	-	-	-	-	-	554	566	-	625	618	-
Stage 2	-	-	-	-	-	-	757	613	-	687	559	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.2			0.4			14.4			12.3		
HCM LOS							B			B		
Minor Lane/Major Mvmt	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2		
Capacity (veh/h)	343	571	1219	-	-	1124	-	-	351	637		
HCM Lane V/C Ratio	0.133	0.088	0.011	-	-	0.014	-	-	0.028	0.038		
HCM Control Delay (s)	17.1	11.9	8	-	-	8.2	-	-	15.6	10.9		
HCM Lane LOS	C	B	A	-	-	A	-	-	C	B		
HCM 95th %tile Q(veh)	0.5	0.3	0	-	-	0	-	-	0.1	0.1		

2: Plaza Dr & Existing Driveway
2024 EXISTING CONDITIONS - PM Peak Hour

HCM 6th TWSC

Intersection						
Int Delay, s/veh	1.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	11	15	5	78	28	9
Future Vol, veh/h	11	15	5	78	28	9
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	12	16	5	85	30	10

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	130	35	40	0	-	0
Stage 1	35	-	-	-	-	-
Stage 2	95	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	864	1038	1570	-	-	-
Stage 1	987	-	-	-	-	-
Stage 2	929	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	861	1038	1570	-	-	-
Mov Cap-2 Maneuver	861	-	-	-	-	-
Stage 1	984	-	-	-	-	-
Stage 2	929	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	8.9	0.4	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1570	-	955	-	-
HCM Lane V/C Ratio	0.003	-	0.03	-	-
HCM Control Delay (s)	7.3	0	8.9	-	-
HCM Lane LOS	A	A	A	-	-
HCM 95th %tile Q(veh)	0	-	0.1	-	-

3: Idaho Rd & Apache Tr

2024 EXISTING CONDITIONS - PM Peak Hour

HCM 6th Signalized Intersection Summary

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	49	43	11	84	51	0	14	304	73	8	354	65
Future Volume (veh/h)	49	43	11	84	51	0	14	304	73	8	354	65
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	53	47	12	91	55	0	15	330	79	9	385	71
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	229	323	79	226	406	0	738	2728	1156	832	2188	400
Arrive On Green	0.11	0.11	0.11	0.11	0.11	0.00	0.73	0.73	0.73	0.73	0.73	0.73
Sat Flow, veh/h	1349	2827	695	1344	3647	0	935	3741	1585	1050	3000	548
Grp Volume(v), veh/h	53	29	30	91	55	0	15	330	79	9	227	229
Grp Sat Flow(s),veh/h/ln	1349	1777	1745	1344	1777	0	935	1870	1585	1050	1777	1772
Q Serve(g_s), s	2.8	1.1	1.2	5.0	1.1	0.0	0.4	2.0	1.1	0.2	3.0	3.1
Cycle Q Clear(g_c), s	3.9	1.1	1.2	6.2	1.1	0.0	3.5	2.0	1.1	2.2	3.0	3.1
Prop In Lane	1.00		0.40	1.00		0.00	1.00		1.00	1.00		0.31
Lane Grp Cap(c), veh/h	229	203	199	226	406	0	738	2728	1156	832	1296	1292
V/C Ratio(X)	0.23	0.14	0.15	0.40	0.14	0.00	0.02	0.12	0.07	0.01	0.17	0.18
Avail Cap(c_a), veh/h	813	972	955	808	1944	0	738	2728	1156	832	1296	1292
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	32.3	30.6	30.6	33.4	30.6	0.0	3.8	3.1	3.0	3.4	3.2	3.2
Incr Delay (d2), s/veh	0.5	0.3	0.3	1.1	0.1	0.0	0.1	0.1	0.1	0.0	0.3	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.9	0.5	0.5	1.7	0.5	0.0	0.1	0.6	0.3	0.0	0.9	0.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	32.8	30.9	31.0	34.6	30.7	0.0	3.8	3.2	3.1	3.4	3.5	3.5
LnGrp LOS	C	C	C	C	C	A	A	A	A	A	A	A
Approach Vol, veh/h	112				146				424			
Approach Delay, s/veh	31.9				33.1				3.2			
Approach LOS	C				C				A			
Timer - Assigned Phs	2			4			6			8		
Phs Duration (G+Y+Rc), s	62.0			14.8			62.0			14.8		
Change Period (Y+Rc), s	6.0			6.0			6.0			6.0		
Max Green Setting (Gmax), s	56.0			42.0			56.0			42.0		
Max Q Clear Time (g_c+I1), s	5.5			5.9			5.1			8.2		
Green Ext Time (p_c), s	2.7			0.5			3.1			0.6		
Intersection Summary												
HCM 6th Ctrl Delay	9.9											
HCM 6th LOS	A											

4: Apache Tr/Phelps Dr & Old West Hwy

2024 EXISTING CONDITIONS - PM Peak Hour

HCM 6th Signalized Intersection Summary



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↰ ↱ ↱ ↱			↰ ↱ ↱ ↱			↰ ↱	↱ ↱		↰ ↱	↱ ↱	↰ ↱
Traffic Volume (veh/h)	54	575	80	62	398	39	127	57	65	47	55	74
Future Volume (veh/h)	54	575	80	62	398	39	127	57	65	47	55	74
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	59	625	87	67	433	42	90	129	71	51	75	70
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	151	943	130	87	1010	97	629	882	457	559	692	586
Arrive On Green	0.04	0.21	0.21	0.05	0.21	0.21	0.05	0.38	0.38	0.04	0.37	0.37
Sat Flow, veh/h	3456	4538	624	1781	4740	453	1781	2322	1203	1781	1870	1585
Grp Volume(v), veh/h	59	467	245	67	309	166	90	102	98	51	75	70
Grp Sat Flow(s),veh/h/ln	1728	1702	1758	1781	1702	1789	1781	1870	1654	1781	1870	1585
Q Serve(g_s), s	1.4	10.9	11.1	3.2	6.8	7.0	2.7	3.1	3.4	1.5	2.3	2.5
Cycle Q Clear(g_c), s	1.4	10.9	11.1	3.2	6.8	7.0	2.7	3.1	3.4	1.5	2.3	2.5
Prop In Lane	1.00		0.36	1.00		0.25	1.00		0.73	1.00		1.00
Lane Grp Cap(c), veh/h	151	708	365	87	725	381	629	711	629	559	692	586
V/C Ratio(X)	0.39	0.66	0.67	0.77	0.43	0.44	0.14	0.14	0.16	0.09	0.11	0.12
Avail Cap(c_a), veh/h	517	1451	749	267	1451	762	743	711	629	671	692	586
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	40.4	31.6	31.7	40.8	29.6	29.6	15.4	17.7	17.7	15.6	18.0	18.0
Incr Delay (d2), s/veh	1.6	1.1	2.1	13.3	0.4	0.8	0.1	0.4	0.5	0.1	0.3	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	4.5	4.8	1.7	2.8	3.0	1.1	1.4	1.3	0.6	1.0	1.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	42.0	32.6	33.8	54.1	30.0	30.4	15.5	18.1	18.3	15.6	18.3	18.5
LnGrp LOS	D	C	C	D	C	C	B	B	B	B	B	B
Approach Vol, veh/h	771		542			290			196			
Approach Delay, s/veh	33.7		33.1			17.3			17.7			
Approach LOS	C		C			B			B			
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	40.5	40.0	11.2	25.0	11.4	39.1	10.8	25.5				
Change Period (Y+Rc), s	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0				
Max Green Setting (Gmax), s	33.0	33.0	13.0	37.0	10.0	32.0	13.0	37.0				
Max Q Clear Time (g_c+I), s	13.5	5.4	5.2	13.1	4.7	4.5	3.4	9.0				
Green Ext Time (p_c), s	0.0	1.2	0.1	4.9	0.1	0.6	0.1	3.2				

Intersection Summary

HCM 6th Ctrl Delay	29.1
HCM 6th LOS	C

Notes

User approved volume balancing among the lanes for turning movement.

APPENDIX E: Trip Generation Calculation

DESCRIPTION OF LAND USE				VEHICLE GENERATED TRIPS									Data Points	AVERAGE RATE or EQUATION			DIRECTIONAL DISTRIBUTION					
				Daily			AM Peak Hour			PM Peak Hour				Daily	AM Peak Hour	PM Peak Hour	Daily		AM Peak Hour		PM Peak Hour	
Phase/Parcel	Land Use	ITE Land Use Code (LUC)	SIZE	Enter	Exit	Total	Enter	Exit	Total	Enter	Exit	Total		Daily	AM Peak Hour	PM Peak Hour	Enter	Exit	Enter	Exit	Enter	Exit
	Multi-family Residential	220 Multifamily Housing	336 DU	1,133	1,132	2,265	32	102	134	108	63	171		6.74	0.4	0.51	50%	50%	24%	76%	63%	37%
Total Trips				1,133	1,132	2,265	32	102	134	108	63	171										
Alternate Mode Reduction 0%				0	0	0	0	0	0	0	0	0										
Internal Trips (Retail/Restaurant) 20%				0	0	0	0	0	0	0	0	0										
"New" Trips Added to Street System				1,133	1,132	2,265	32	102	134	108	63	171										

Source: ITE Trip Generation Manual 11th Edition

APPENDIX F: Future 2026 Level-of-Service Analysis

1: Site Access A/Plaza Dr & Superstition Blvd
2026 BACKGROUND ONLY- AM Peak Hour

HCM 6th TWSC

Intersection												
Int Delay, s/veh	1.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	15	250	72	33	270	6	21	3	15	9	6	10
Future Vol, veh/h	15	250	72	33	270	6	21	3	15	9	6	10
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	0	-	-	0	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	16	272	78	36	293	7	23	3	16	10	7	11

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	300	0	0	350	0	0	565	715	175	539	751	150
Stage 1	-	-	-	-	-	-	343	343	-	369	369	-
Stage 2	-	-	-	-	-	-	222	372	-	170	382	-
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32
Pot Cap-1 Maneuver	1258	-	-	1206	-	-	408	355	838	426	338	870
Stage 1	-	-	-	-	-	-	646	636	-	623	619	-
Stage 2	-	-	-	-	-	-	760	617	-	815	611	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1258	-	-	1206	-	-	384	340	838	401	323	870
Mov Cap-2 Maneuver	-	-	-	-	-	-	384	340	-	401	323	-
Stage 1	-	-	-	-	-	-	638	628	-	615	600	-
Stage 2	-	-	-	-	-	-	720	598	-	785	603	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.4			0.9			12.9			12.8		
HCM LOS							B			B		

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)	384	674	1258	-	-	1206	-	-	401	532
HCM Lane V/C Ratio	0.059	0.029	0.013	-	-	0.03	-	-	0.024	0.033
HCM Control Delay (s)	15	10.5	7.9	-	-	8.1	-	-	14.2	12
HCM Lane LOS	C	B	A	-	-	A	-	-	B	B
HCM 95th %tile Q(veh)	0.2	0.1	0	-	-	0.1	-	-	0.1	0.1

2: Plaza Dr & Existing Driveway
2026 BACKGROUND ONLY- AM Peak Hour

HCM 6th TWSC

Intersection						
Int Delay, s/veh	1.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	12	18	8	26	109	4
Future Vol, veh/h	12	18	8	26	109	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	13	20	9	28	118	4

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	166	120	122	0	-	0
Stage 1	120	-	-	-	-	-
Stage 2	46	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	824	931	1465	-	-	-
Stage 1	905	-	-	-	-	-
Stage 2	976	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	819	931	1465	-	-	-
Mov Cap-2 Maneuver	819	-	-	-	-	-
Stage 1	900	-	-	-	-	-
Stage 2	976	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	9.2	1.8	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1465	-	883	-	-
HCM Lane V/C Ratio	0.006	-	0.037	-	-
HCM Control Delay (s)	7.5	0	9.2	-	-
HCM Lane LOS	A	A	A	-	-
HCM 95th %tile Q(veh)	0	-	0.1	-	-

3: Idaho Rd & Apache Tr

2026 BACKGROUND ONLY- AM Peak Hour

HCM 6th Signalized Intersection Summary

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	46	33	9	68	43	3	13	289	99	3	315	61
Future Volume (veh/h)	46	33	9	68	43	3	13	289	99	3	315	61
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	50	36	10	74	47	3	14	314	108	3	342	66
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	531	462	123	532	565	36	523	1116	473	547	888	169
Arrive On Green	0.17	0.17	0.17	0.17	0.17	0.17	0.30	0.30	0.30	0.30	0.30	0.30
Sat Flow, veh/h	1355	2775	739	1360	3394	215	978	3741	1585	965	2977	568
Grp Volume(v), veh/h	50	22	24	74	24	26	14	314	108	3	203	205
Grp Sat Flow(s),veh/h/ln	1355	1777	1737	1360	1777	1832	978	1870	1585	965	1777	1768
Q Serve(g_s), s	0.7	0.2	0.3	1.1	0.3	0.3	0.3	1.4	1.2	0.1	2.0	2.1
Cycle Q Clear(g_c), s	1.0	0.2	0.3	1.3	0.3	0.3	2.3	1.4	1.2	1.5	2.0	2.1
Prop In Lane	1.00		0.43	1.00		0.12	1.00		1.00	1.00		0.32
Lane Grp Cap(c), veh/h	531	296	289	532	296	305	523	1116	473	547	530	527
V/C Ratio(X)	0.09	0.08	0.08	0.14	0.08	0.08	0.03	0.28	0.23	0.01	0.38	0.39
Avail Cap(c_a), veh/h	2421	2774	2713	2429	2774	2860	2106	7175	3040	2110	3408	3392
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	8.3	7.9	7.9	8.5	7.9	7.9	7.2	6.0	5.9	6.6	6.2	6.2
Incr Delay (d2), s/veh	0.1	0.1	0.1	0.1	0.1	0.1	0.0	0.1	0.2	0.0	0.5	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.1	0.1	0.1	0.2	0.1	0.1	0.0	0.2	0.1	0.0	0.2	0.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	8.4	8.0	8.0	8.6	8.0	8.0	7.2	6.2	6.2	6.6	6.7	6.7
LnGrp LOS	A	A	A	A	A	A	A	A	A	A	A	A
Approach Vol, veh/h	96			124			436			411		
Approach Delay, s/veh	8.2			8.4			6.2			6.7		
Approach LOS	A			A			A			A		
Timer - Assigned Phs	2			4			6			8		
Phs Duration (G+Y+Rc), s	12.7			9.7			12.7			9.7		
Change Period (Y+Rc), s	6.0			6.0			6.0			6.0		
Max Green Setting (Gmax), s	43.0			35.0			43.0			35.0		
Max Q Clear Time (g_c+I1), s	4.3			3.0			4.1			3.3		
Green Ext Time (p_c), s	2.4			0.3			2.3			0.4		
Intersection Summary												
HCM 6th Ctrl Delay	6.8											
HCM 6th LOS	A											

4: Apache Tr/Phelps Dr & Old West Hwy
2026 BACKGROUND ONLY- AM Peak Hour

HCM 6th Signalized Intersection Summary



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↰ ↱ ↲ ↳			↰ ↱ ↲ ↳			↰ ↱ ↳			↰ ↱ ↳ ↲		
Traffic Volume (veh/h)	61	400	45	21	466	31	56	50	25	31	30	76
Future Volume (veh/h)	61	400	45	21	466	31	56	50	25	31	30	76
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	66	435	49	23	507	34	47	73	27	34	70	58
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	164	881	98	44	814	54	642	1093	384	658	759	643
Arrive On Green	0.05	0.19	0.19	0.02	0.17	0.17	0.04	0.41	0.41	0.03	0.41	0.41
Sat Flow, veh/h	3456	4664	517	1781	4891	325	1781	2645	929	1781	1870	1585
Grp Volume(v), veh/h	66	316	168	23	352	189	47	50	50	34	70	58
Grp Sat Flow(s),veh/h/ln	1728	1702	1777	1781	1702	1812	1781	1870	1703	1781	1870	1585
Q Serve(g_s), s	1.5	6.8	7.0	1.0	7.9	8.0	1.2	1.3	1.4	0.9	1.9	1.9
Cycle Q Clear(g_c), s	1.5	6.8	7.0	1.0	7.9	8.0	1.2	1.3	1.4	0.9	1.9	1.9
Prop In Lane	1.00		0.29	1.00		0.18	1.00		0.55	1.00		1.00
Lane Grp Cap(c), veh/h	164	643	336	44	567	302	642	773	704	658	759	643
V/C Ratio(X)	0.40	0.49	0.50	0.52	0.62	0.63	0.07	0.07	0.07	0.05	0.09	0.09
Avail Cap(c_a), veh/h	462	1572	821	238	1572	837	766	773	704	795	773	655
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	38.1	29.8	29.9	39.6	31.9	31.9	13.0	14.6	14.6	13.2	15.1	15.1
Incr Delay (d2), s/veh	1.6	0.6	1.2	9.1	1.1	2.1	0.0	0.2	0.2	0.0	0.1	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.7	2.8	3.0	0.6	3.3	3.6	0.5	0.6	0.6	0.3	0.8	0.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	39.7	30.4	31.1	48.8	33.0	34.1	13.0	14.7	14.8	13.2	15.1	15.1
LnGrp LOS	D	C	C	D	C	C	B	B	B	B	B	B
Approach Vol, veh/h	550			564			147			162		
Approach Delay, s/veh	31.7			34.0			14.2			14.7		
Approach LOS	C			C			B			B		
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	9.7	41.0	9.0	22.6	10.3	40.4	10.9	20.7				
Change Period (Y+Rc), s	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0				
Max Green Setting (Gmax), s	9.0	34.0	11.0	38.0	9.0	34.0	11.0	38.0				
Max Q Clear Time (g_c+1.2), s	12.5	3.4	3.0	9.0	3.2	3.9	3.5	10.0				
Green Ext Time (p_c), s	0.0	0.5	0.0	3.3	0.0	0.5	0.1	3.7				

Intersection Summary

HCM 6th Ctrl Delay	28.9
HCM 6th LOS	C

Notes

User approved volume balancing among the lanes for turning movement.

1: Site Access A/Plaza Dr & Superstition Blvd
2026 BACKGROUND ONLY - PM Peak Hour

HCM 6th TWSC

Intersection												
Int Delay, s/veh	2.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	13	373	22	15	272	18	46	12	39	10	4	20
Future Vol, veh/h	13	373	22	15	272	18	46	12	39	10	4	20
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	0	-	-	0	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	14	405	24	16	296	20	50	13	42	11	4	22

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	316	0	0	429	0	0	627	793	215	575	795	158
Stage 1	-	-	-	-	-	-	445	445	-	338	338	-
Stage 2	-	-	-	-	-	-	182	348	-	237	457	-
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32
Pot Cap-1 Maneuver	1241	-	-	1127	-	-	368	320	790	401	319	859
Stage 1	-	-	-	-	-	-	562	573	-	650	639	-
Stage 2	-	-	-	-	-	-	802	633	-	745	566	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1241	-	-	1127	-	-	348	312	790	360	311	859
Mov Cap-2 Maneuver	-	-	-	-	-	-	348	312	-	360	311	-
Stage 1	-	-	-	-	-	-	556	567	-	643	630	-
Stage 2	-	-	-	-	-	-	765	624	-	681	560	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0.4			14.3			12		
HCM LOS							B			B		

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)	348	581	1241	-	-	1127	-	-	360	664
HCM Lane V/C Ratio	0.144	0.095	0.011	-	-	0.014	-	-	0.03	0.039
HCM Control Delay (s)	17.1	11.8	7.9	-	-	8.2	-	-	15.3	10.6
HCM Lane LOS	C	B	A	-	-	A	-	-	C	B
HCM 95th %tile Q(veh)	0.5	0.3	0	-	-	0	-	-	0.1	0.1

2: Plaza Dr & Existing Driveway
2026 BACKGROUND ONLY - PM Peak Hour

HCM 6th TWSC

Intersection						
Int Delay, s/veh	1.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	12	17	6	86	31	10
Future Vol, veh/h	12	17	6	86	31	10
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	13	18	7	93	34	11

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	147	40	45	0	-	0
Stage 1	40	-	-	-	-	-
Stage 2	107	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	845	1031	1563	-	-	-
Stage 1	982	-	-	-	-	-
Stage 2	917	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	841	1031	1563	-	-	-
Mov Cap-2 Maneuver	841	-	-	-	-	-
Stage 1	977	-	-	-	-	-
Stage 2	917	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	9	0.5	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1563	-	943	-	-
HCM Lane V/C Ratio	0.004	-	0.033	-	-
HCM Control Delay (s)	7.3	0	9	-	-
HCM Lane LOS	A	A	A	-	-
HCM 95th %tile Q(veh)	0	-	0.1	-	-

3: Idaho Rd & Apache Tr

2026 BACKGROUND ONLY - PM Peak Hour

HCM 6th Signalized Intersection Summary

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	54	47	12	93	56	0	15	335	80	9	390	72
Future Volume (veh/h)	54	47	12	93	56	0	15	335	80	9	390	72
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	59	51	13	101	61	0	16	364	87	10	424	78
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	238	350	86	236	440	0	698	2700	1144	797	2165	395
Arrive On Green	0.12	0.12	0.12	0.12	0.12	0.00	0.72	0.72	0.72	0.72	0.72	0.72
Sat Flow, veh/h	1341	2828	694	1338	3647	0	896	3741	1585	1018	3001	548
Grp Volume(v), veh/h	59	31	33	101	61	0	16	364	87	10	250	252
Grp Sat Flow(s),veh/h/ln	1341	1777	1745	1338	1777	0	896	1870	1585	1018	1777	1772
Q Serve(g_s), s	3.2	1.2	1.3	5.7	1.2	0.0	0.5	2.3	1.3	0.2	3.5	3.6
Cycle Q Clear(g_c), s	4.4	1.2	1.3	7.0	1.2	0.0	4.0	2.3	1.3	2.6	3.5	3.6
Prop In Lane	1.00		0.40	1.00		0.00	1.00		1.00	1.00		0.31
Lane Grp Cap(c), veh/h	238	220	216	236	440	0	698	2700	1144	797	1282	1279
V/C Ratio(X)	0.25	0.14	0.15	0.43	0.14	0.00	0.02	0.13	0.08	0.01	0.19	0.20
Avail Cap(c_a), veh/h	798	962	945	795	1923	0	698	2700	1144	797	1282	1279
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	32.3	30.3	30.4	33.5	30.3	0.0	4.2	3.3	3.2	3.7	3.5	3.5
Incr Delay (d2), s/veh	0.5	0.3	0.3	1.2	0.1	0.0	0.1	0.1	0.1	0.0	0.3	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.0	0.5	0.6	1.9	0.5	0.0	0.1	0.7	0.3	0.0	1.0	1.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	32.8	30.6	30.7	34.7	30.5	0.0	4.2	3.4	3.3	3.8	3.8	3.9
LnGrp LOS	C	C	C	C	C	A	A	A	A	A	A	A
Approach Vol, veh/h	123			162			467			512		
Approach Delay, s/veh	31.7			33.1			3.4			3.8		
Approach LOS	C			C			A			A		
Timer - Assigned Phs	2			4			6			8		
Phs Duration (G+Y+Rc), s	62.0			15.6			62.0			15.6		
Change Period (Y+Rc), s	6.0			6.0			6.0			6.0		
Max Green Setting (Gmax), s	56.0			42.0			56.0			42.0		
Max Q Clear Time (g_c+I1), s	6.0			6.4			5.6			9.0		
Green Ext Time (p_c), s	3.1			0.5			3.5			0.7		
Intersection Summary												
HCM 6th Ctrl Delay	10.2											
HCM 6th LOS	B											

4: Apache Tr/Phelps Dr & Old West Hwy
2026 BACKGROUND ONLY - PM Peak Hour

HCM 6th Signalized Intersection Summary



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↰ ↱ ↑↑↓			↰ ↱ ↑↑↓			↰ ↱ ↑↑↓			↰ ↱ ↑↑↓		
Traffic Volume (veh/h)	60	634	88	68	439	43	140	63	72	52	61	82
Future Volume (veh/h)	60	634	88	68	439	43	140	63	72	52	61	82
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	65	689	96	74	477	47	99	142	78	57	83	78
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	155	1012	140	96	1100	107	606	854	443	532	669	567
Arrive On Green	0.04	0.22	0.22	0.05	0.23	0.23	0.05	0.37	0.37	0.04	0.36	0.36
Sat Flow, veh/h	3456	4536	626	1781	4731	460	1781	2321	1203	1781	1870	1585
Grp Volume(v), veh/h	65	515	270	74	341	183	99	113	107	57	83	78
Grp Sat Flow(s),veh/h/ln	1728	1702	1758	1781	1702	1788	1781	1870	1654	1781	1870	1585
Q Serve(g_s), s	1.6	12.4	12.6	3.7	7.7	7.8	3.1	3.6	3.9	1.8	2.7	3.0
Cycle Q Clear(g_c), s	1.6	12.4	12.6	3.7	7.7	7.8	3.1	3.6	3.9	1.8	2.7	3.0
Prop In Lane	1.00		0.36	1.00		0.26	1.00		0.73	1.00		1.00
Lane Grp Cap(c), veh/h	155	760	392	96	791	416	606	689	609	532	669	567
V/C Ratio(X)	0.42	0.68	0.69	0.77	0.43	0.44	0.16	0.16	0.18	0.11	0.12	0.14
Avail Cap(c_a), veh/h	501	1405	726	258	1405	738	710	689	609	636	669	567
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	41.7	31.9	31.9	41.8	29.3	29.4	16.5	19.0	19.1	16.7	19.4	19.5
Incr Delay (d2), s/veh	1.8	1.1	2.1	12.1	0.4	0.7	0.1	0.5	0.6	0.1	0.4	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.7	5.1	5.5	1.9	3.1	3.4	1.3	1.6	1.6	0.7	1.2	1.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	43.5	32.9	34.1	54.0	29.7	30.1	16.7	19.6	19.8	16.8	19.7	20.0
LnGrp LOS	D	C	C	D	C	C	B	B	B	B	B	B
Approach Vol, veh/h	850		598			319			218			
Approach Delay, s/veh	34.1		32.8			18.7			19.1			
Approach LOS	C		C			B			B			
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	40.8	40.0	11.8	27.0	11.8	39.0	11.0	27.8				
Change Period (Y+Rc), s	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0				
Max Green Setting (Gmax), s	33.0	33.0	13.0	37.0	10.0	32.0	13.0	37.0				
Max Q Clear Time (g_c+I), s	13.8	5.9	5.7	14.6	5.1	5.0	3.6	9.8				
Green Ext Time (p_c), s	0.0	1.3	0.1	5.4	0.1	0.7	0.1	3.6				
Intersection Summary												
HCM 6th Ctrl Delay			29.6									
HCM 6th LOS			C									
Notes												

1: Site Access A/Plaza Dr & Superstition Blvd
2026 TOTAL (Background plus Site Full Build-out) - AM Peak Hour

HCM 6th TWSC

Intersection												
Int Delay, s/veh	2.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↰	↑↱		↰	↑↱		↰	↑↱		↰	↑↱	
Traffic Vol, veh/h	15	250	81	34	270	6	52	5	18	9	7	10
Future Vol, veh/h	15	250	81	34	270	6	52	5	18	9	7	10
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	0	-	-	0	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	16	272	88	37	293	7	57	5	20	10	8	11
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	300	0	0	360	0	0	573	722	180	542	763	150
Stage 1	-	-	-	-	-	-	348	348	-	371	371	-
Stage 2	-	-	-	-	-	-	225	374	-	171	392	-
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32
Pot Cap-1 Maneuver	1258	-	-	1195	-	-	402	351	832	423	333	870
Stage 1	-	-	-	-	-	-	641	633	-	622	618	-
Stage 2	-	-	-	-	-	-	757	616	-	814	605	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1258	-	-	1195	-	-	377	336	832	395	318	870
Mov Cap-2 Maneuver	-	-	-	-	-	-	377	336	-	395	318	-
Stage 1	-	-	-	-	-	-	633	625	-	614	599	-
Stage 2	-	-	-	-	-	-	715	597	-	778	597	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0.9			14.6			13.1		
HCM LOS							B			B		
Minor Lane/Major Mvmt	NBLn1		NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2	
Capacity (veh/h)	377		630	1258	-	-	1195	-	-	395	507	
HCM Lane V/C Ratio	0.15		0.04	0.013	-	-	0.031	-	-	0.025	0.036	
HCM Control Delay (s)	16.2		11	7.9	-	-	8.1	-	-	14.3	12.4	
HCM Lane LOS	C		B	A	-	-	A	-	-	B	B	
HCM 95th %tile Q(veh)	0.5		0.1	0	-	-	0.1	-	-	0.1	0.1	

2: Plaza Dr & Existing Driveway

2026 TOTAL (Background plus Site Full Build-out) - AM Peak Hour

HCM 6th TWSC

Intersection						
Int Delay, s/veh	1.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	12	18	8	26	109	4
Future Vol, veh/h	12	18	8	26	109	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	13	20	9	28	118	4
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	166	120	122	0	-	0
Stage 1	120	-	-	-	-	-
Stage 2	46	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	824	931	1465	-	-	-
Stage 1	905	-	-	-	-	-
Stage 2	976	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	819	931	1465	-	-	-
Mov Cap-2 Maneuver	819	-	-	-	-	-
Stage 1	900	-	-	-	-	-
Stage 2	976	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	9.2	1.8		0		
HCM LOS	A					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	1465	-	883	-	-	
HCM Lane V/C Ratio	0.006	-	0.037	-	-	
HCM Control Delay (s)	7.5	0	9.2	-	-	
HCM Lane LOS	A	A	A	-	-	
HCM 95th %tile Q(veh)	0	-	0.1	-	-	

3: Idaho Rd & Apache Tr

2026 TOTAL (Background plus Site Full Build-out) - AM Peak Hour HCM 6th Signalized Intersection Summary

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	53	41	30	68	46	3	19	289	99	3	315	63
Future Volume (veh/h)	53	41	30	68	46	3	19	289	99	3	315	63
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	58	45	33	74	50	3	21	314	108	3	342	68
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	536	362	240	520	603	36	517	1127	477	542	892	175
Arrive On Green	0.18	0.18	0.18	0.18	0.18	0.18	0.30	0.30	0.30	0.30	0.30	0.30
Sat Flow, veh/h	1351	2046	1357	1321	3408	203	976	3741	1585	965	2960	582
Grp Volume(v), veh/h	58	38	40	74	26	27	21	314	108	3	204	206
Grp Sat Flow(s),veh/h/ln	1351	1777	1626	1321	1777	1834	976	1870	1585	965	1777	1766
Q Serve(g_s), s	0.9	0.4	0.5	1.2	0.3	0.3	0.4	1.5	1.2	0.1	2.1	2.1
Cycle Q Clear(g_c), s	1.1	0.4	0.5	1.6	0.3	0.3	2.5	1.5	1.2	1.5	2.1	2.1
Prop In Lane	1.00		0.83	1.00		0.11	1.00		1.00	1.00		0.33
Lane Grp Cap(c), veh/h	536	314	288	520	314	324	517	1127	477	542	535	532
V/C Ratio(X)	0.11	0.12	0.14	0.14	0.08	0.08	0.04	0.28	0.23	0.01	0.38	0.39
Avail Cap(c_a), veh/h	2353	2705	2475	2297	2705	2792	2048	6996	2964	2056	3323	3302
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	8.4	8.0	8.0	8.7	7.9	7.9	7.4	6.1	6.0	6.7	6.3	6.4
Incr Delay (d2), s/veh	0.1	0.2	0.2	0.1	0.1	0.1	0.0	0.1	0.2	0.0	0.4	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.1	0.1	0.1	0.2	0.1	0.1	0.0	0.2	0.1	0.0	0.3	0.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	8.5	8.1	8.2	8.8	8.0	8.0	7.4	6.3	6.3	6.7	6.8	6.8
LnGrp LOS	A	A	A	A	A	A	A	A	A	A	A	A
Approach Vol, veh/h	136			127			443			413		
Approach Delay, s/veh	8.3			8.5			6.3			6.8		
Approach LOS	A			A			A			A		
Timer - Assigned Phs	2			4			6			8		
Phs Duration (G+Y+Rc), s	12.9			10.1			12.9			10.1		
Change Period (Y+Rc), s	6.0			6.0			6.0			6.0		
Max Green Setting (Gmax), s	43.0			35.0			43.0			35.0		
Max Q Clear Time (g_c+l1), s	4.5			3.1			4.1			3.6		
Green Ext Time (p_c), s	2.4			0.6			2.3			0.5		
Intersection Summary												
HCM 6th Ctrl Delay	7.0											
HCM 6th LOS	A											
Notes												

4: Apache Tr/Phelps Dr & Old West Hwy

2026 TOTAL (Background plus Site Full Build-out) - AM Peak Hour HCM 6th Signalized Intersection Summary



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↰ ↱ ↑↑↑			↰ ↱ ↑↑↑			↰ ↱			↰ ↱		↰ ↱
Traffic Volume (veh/h)	67	400	45	21	466	33	56	52	25	37	35	96
Future Volume (veh/h)	67	400	45	21	466	33	56	52	25	37	35	96
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	73	435	49	23	507	36	48	75	27	40	88	71
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	170	891	99	44	812	57	621	1093	374	658	760	644
Arrive On Green	0.05	0.19	0.19	0.02	0.17	0.17	0.04	0.41	0.41	0.04	0.41	0.41
Sat Flow, veh/h	3456	4664	517	1781	4870	343	1781	2665	912	1781	1870	1585
Grp Volume(v), veh/h	73	316	168	23	353	190	48	51	51	40	88	71
Grp Sat Flow(s),veh/h/ln	1728	1702	1777	1781	1702	1809	1781	1870	1706	1781	1870	1585
Q Serve(g_s), s	1.7	6.9	7.0	1.1	8.0	8.1	1.3	1.4	1.5	1.1	2.4	2.3
Cycle Q Clear(g_c), s	1.7	6.9	7.0	1.1	8.0	8.1	1.3	1.4	1.5	1.1	2.4	2.3
Prop In Lane	1.00		0.29	1.00		0.19	1.00		0.53	1.00		1.00
Lane Grp Cap(c), veh/h	170	650	339	44	567	301	621	767	700	658	760	644
V/C Ratio(X)	0.43	0.49	0.50	0.52	0.62	0.63	0.08	0.07	0.07	0.06	0.12	0.11
Avail Cap(c_a), veh/h	459	1560	815	236	1560	829	743	767	700	787	767	650
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	38.3	29.9	30.0	39.9	32.1	32.2	13.1	14.8	14.9	13.2	15.3	15.3
Incr Delay (d2), s/veh	1.7	0.6	1.1	9.2	1.1	2.2	0.1	0.2	0.2	0.0	0.1	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.7	2.8	3.0	0.6	3.3	3.6	0.5	0.6	0.6	0.4	1.0	0.8
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	40.0	30.5	31.1	49.1	33.2	34.3	13.1	15.0	15.1	13.2	15.4	15.4
LnGrp LOS	D	C	C	D	C	C	B	B	B	B	B	B
Approach Vol, veh/h	557			566			150			199		
Approach Delay, s/veh	31.9			34.2			14.4			15.0		
Approach LOS	C			C			B			B		
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	41.0	41.0	9.1	22.8	10.3	40.7	11.1	20.8				
Change Period (Y+Rc), s	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0				
Max Green Setting (Gmax), s	34.0	34.0	11.0	38.0	9.0	34.0	11.0	38.0				
Max Q Clear Time (g_c+I1), s	3.5	3.5	3.1	9.0	3.3	4.4	3.7	10.1				
Green Ext Time (p_c), s	0.0	0.5	0.0	3.3	0.0	0.7	0.1	3.7				

Intersection Summary

HCM 6th Ctrl Delay 28.7

HCM 6th LOS C

Notes

User approved volume balancing among the lanes for turning movement.

5: Apache Tr & Site Access B

2026 TOTAL (Background plus Site Full Build-out) - AM Peak Hour

HCM 6th TWSC

Intersection						
Int Delay, s/veh	2.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	36	31	10	88	118	11
Future Vol, veh/h	36	31	10	88	118	11
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	39	34	11	96	128	12
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	252	134	140	0	-	0
Stage 1	134	-	-	-	-	-
Stage 2	118	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	737	915	1443	-	-	-
Stage 1	892	-	-	-	-	-
Stage 2	907	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	731	915	1443	-	-	-
Mov Cap-2 Maneuver	731	-	-	-	-	-
Stage 1	885	-	-	-	-	-
Stage 2	907	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	9.7	0.8		0		
HCM LOS	A					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	1443	-	731	915	-	-
HCM Lane V/C Ratio	0.008	-	0.054	0.037	-	-
HCM Control Delay (s)	7.5	0	10.2	9.1	-	-
HCM Lane LOS	A	A	B	A	-	-
HCM 95th %tile Q(veh)	0	-	0.2	0.1	-	-

6: Plaza Dr & Site Access A

2026 TOTAL (Background plus Site Full Build-out) - AM Peak Hour

HCM 6th TWSC

Intersection						
Int Delay, s/veh	1.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	36	39	0	11	114
Future Vol, veh/h	0	36	39	0	11	114
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	39	42	0	12	124
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	190	42	0	0	42	0
Stage 1	42	-	-	-	-	-
Stage 2	148	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	799	1029	-	-	1567	-
Stage 1	980	-	-	-	-	-
Stage 2	880	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	793	1029	-	-	1567	-
Mov Cap-2 Maneuver	793	-	-	-	-	-
Stage 1	980	-	-	-	-	-
Stage 2	873	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	8.6	0		0.6		
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	- 1029	1567	-		
HCM Lane V/C Ratio	-	- 0.038	0.008	-		
HCM Control Delay (s)	-	- 8.6	7.3	0		
HCM Lane LOS	-	- A	A	A		
HCM 95th %tile Q(veh)	-	- 0.1	0	-		

1: Site Access A/Plaza Dr & Superstition Blvd
2026 TOTAL (Background plus Site Full Build-out) - PM Peak Hour

HCM 6th TWSC

Intersection												
Int Delay, s/veh	2.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	13	373	55	18	272	18	65	13	41	10	6	20
Future Vol, veh/h	13	373	55	18	272	18	65	13	41	10	6	20
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	0	-	-	0	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	14	405	60	20	296	20	71	14	45	11	7	22

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	316	0	0	465	0	0	655	819	233	584	839	158
Stage 1	-	-	-	-	-	-	463	463	-	346	346	-
Stage 2	-	-	-	-	-	-	192	356	-	238	493	-
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32
Pot Cap-1 Maneuver	1241	-	-	1093	-	-	351	309	769	395	300	859
Stage 1	-	-	-	-	-	-	548	562	-	643	634	-
Stage 2	-	-	-	-	-	-	791	628	-	744	545	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1241	-	-	1093	-	-	329	300	769	351	291	859
Mov Cap-2 Maneuver	-	-	-	-	-	-	329	300	-	351	291	-
Stage 1	-	-	-	-	-	-	542	556	-	636	623	-
Stage 2	-	-	-	-	-	-	749	617	-	675	539	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.2			0.5			15.9			12.6		
HCM LOS							C			B		

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)	329	559	1241	-	-	1093	-	-	351	592
HCM Lane V/C Ratio	0.215	0.105	0.011	-	-	0.018	-	-	0.031	0.048
HCM Control Delay (s)	18.9	12.2	7.9	-	-	8.4	-	-	15.6	11.4
HCM Lane LOS	C	B	A	-	-	A	-	-	C	B
HCM 95th %tile Q(veh)	0.8	0.3	0	-	-	0.1	-	-	0.1	0.1

2: Plaza Dr & Existing Driveway

2026 TOTAL (Background plus Site Full Build-out) - PM Peak Hour

HCM 6th TWSC

Intersection						
Int Delay, s/veh	1.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	12	17	6	86	31	10
Future Vol, veh/h	12	17	6	86	31	10
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	13	18	7	93	34	11
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	147	40	45	0	-	0
Stage 1	40	-	-	-	-	-
Stage 2	107	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	845	1031	1563	-	-	-
Stage 1	982	-	-	-	-	-
Stage 2	917	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	841	1031	1563	-	-	-
Mov Cap-2 Maneuver	841	-	-	-	-	-
Stage 1	977	-	-	-	-	-
Stage 2	917	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	9	0.5		0		
HCM LOS	A					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	1563	-	943	-	-	
HCM Lane V/C Ratio	0.004	-	0.033	-	-	
HCM Control Delay (s)	7.3	0	9	-	-	
HCM Lane LOS	A	A	A	-	-	
HCM 95th %tile Q(veh)	0	-	0.1	-	-	

3: Idaho Rd & Apache Tr

2026 TOTAL (Background plus Site Full Build-out) - PM Peak Hour HCM 6th Signalized Intersection Summary

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	58	52	25	93	65	0	37	335	80	9	390	79
Future Volume (veh/h)	58	52	25	93	65	0	37	335	80	9	390	79
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	63	57	27	101	71	0	40	364	87	10	424	86
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	243	315	140	235	468	0	685	2675	1133	789	2108	424
Arrive On Green	0.13	0.13	0.13	0.13	0.13	0.00	0.72	0.72	0.72	0.72	0.72	0.72
Sat Flow, veh/h	1329	2394	1062	1314	3647	0	890	3741	1585	1018	2947	593
Grp Volume(v), veh/h	63	41	43	101	71	0	40	364	87	10	254	256
Grp Sat Flow(s),veh/h/ln	1329	1777	1679	1314	1777	0	890	1870	1585	1018	1777	1764
Q Serve(g_s), s	3.5	1.6	1.8	5.8	1.4	0.0	1.2	2.4	1.3	0.2	3.7	3.8
Cycle Q Clear(g_c), s	4.8	1.6	1.8	7.6	1.4	0.0	5.0	2.4	1.3	2.7	3.7	3.8
Prop In Lane	1.00		0.63	1.00		0.00	1.00		1.00	1.00		0.34
Lane Grp Cap(c), veh/h	243	234	221	235	468	0	685	2675	1133	789	1271	1261
V/C Ratio(X)	0.26	0.18	0.19	0.43	0.15	0.00	0.06	0.14	0.08	0.01	0.20	0.20
Avail Cap(c_a), veh/h	781	953	901	767	1906	0	685	2675	1133	789	1271	1261
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	32.3	30.2	30.3	33.7	30.1	0.0	4.6	3.5	3.4	3.9	3.7	3.7
Incr Delay (d2), s/veh	0.6	0.4	0.4	1.2	0.1	0.0	0.2	0.1	0.1	0.0	0.4	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.1	0.7	0.7	1.9	0.6	0.0	0.2	0.7	0.3	0.0	1.1	1.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	32.8	30.6	30.7	34.9	30.3	0.0	4.7	3.6	3.5	4.0	4.1	4.1
LnGrp LOS	C	C	C	C	C	A	A	A	A	A	A	A
Approach Vol, veh/h	147				172				491			
Approach Delay, s/veh	31.6				33.0				3.7			
Approach LOS	C				C				A			
Timer - Assigned Phs	2			4			6			8		
Phs Duration (G+Y+Rc), s	62.0			16.3			62.0			16.3		
Change Period (Y+Rc), s	6.0			6.0			6.0			6.0		
Max Green Setting (Gmax), s	56.0			42.0			56.0			42.0		
Max Q Clear Time (g_c+l1), s	7.0			6.8			5.8			9.6		
Green Ext Time (p_c), s	3.2			0.7			3.5			0.7		
Intersection Summary												
HCM 6th Ctrl Delay	10.7											
HCM 6th LOS	B											
Notes												

4: Apache Tr/Phelps Dr & Old West Hwy

2026 TOTAL (Background plus Site Full Build-out) - PM Peak Hour HCM 6th Signalized Intersection Summary



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	RT		RT		RT		RT		RT		RT	
Traffic Volume (veh/h)	82	634	88	68	439	48	140	68	72	55	64	95
Future Volume (veh/h)	82	634	88	68	439	48	140	68	72	55	64	95
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	89	689	96	74	477	52	101	145	78	60	95	86
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	172	1012	140	96	1065	114	596	860	437	532	668	566
Arrive On Green	0.05	0.22	0.22	0.05	0.23	0.23	0.05	0.37	0.37	0.04	0.36	0.36
Sat Flow, veh/h	3456	4536	626	1781	4681	503	1781	2338	1189	1781	1870	1585
Grp Volume(v), veh/h	89	515	270	74	345	184	101	114	109	60	95	86
Grp Sat Flow(s),veh/h/ln	1728	1702	1758	1781	1702	1780	1781	1870	1656	1781	1870	1585
Q Serve(g_s), s	2.3	12.4	12.6	3.7	7.8	8.0	3.2	3.7	4.0	1.9	3.1	3.3
Cycle Q Clear(g_c), s	2.3	12.4	12.6	3.7	7.8	8.0	3.2	3.7	4.0	1.9	3.1	3.3
Prop In Lane	1.00		0.36	1.00		0.28	1.00		0.72	1.00		1.00
Lane Grp Cap(c), veh/h	172	759	392	96	774	405	596	688	609	532	668	566
V/C Ratio(X)	0.52	0.68	0.69	0.77	0.45	0.45	0.17	0.17	0.18	0.11	0.14	0.15
Avail Cap(c_a), veh/h	501	1403	725	258	1403	734	698	688	609	633	668	566
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	41.6	31.9	32.0	41.9	29.8	29.9	16.6	19.1	19.2	16.8	19.5	19.6
Incr Delay (d2), s/veh	2.4	1.1	2.1	12.1	0.4	0.8	0.1	0.5	0.6	0.1	0.4	0.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.0	5.1	5.5	1.9	3.2	3.5	1.3	1.7	1.6	0.8	1.4	1.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	44.0	33.0	34.1	54.0	30.2	30.7	16.7	19.6	19.8	16.8	20.0	20.2
LnGrp LOS	D	C	C	D	C	C	B	B	B	B	B	C
Approach Vol, veh/h	874		603			324			241			
Approach Delay, s/veh	34.5		33.3			18.8			19.3			
Approach LOS	C		C			B			B			
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	40.9	40.0	11.8	27.0	11.8	39.0	11.5	27.4				
Change Period (Y+Rc), s	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0				
Max Green Setting (Gmax), s	33.0	33.0	13.0	37.0	10.0	32.0	13.0	37.0				
Max Q Clear Time (g_c+I), s	6.0	6.0	5.7	14.6	5.2	5.3	4.3	10.0				
Green Ext Time (p_c), s	0.0	1.3	0.1	5.4	0.1	0.8	0.1	3.6				

Intersection Summary

HCM 6th Ctrl Delay 29.8

HCM 6th LOS C

Notes

User approved volume balancing among the lanes for turning movement.

5: Apache Tr & Site Access B

2026 TOTAL (Background plus Site Full Build-out) - PM Peak Hour

HCM 6th TWSC

Intersection						
Int Delay, s/veh	1.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	22	19	32	114	143	38
Future Vol, veh/h	22	19	32	114	143	38
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	24	21	35	124	155	41
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	370	176	196	0	-	0
Stage 1	176	-	-	-	-	-
Stage 2	194	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	630	867	1377	-	-	-
Stage 1	855	-	-	-	-	-
Stage 2	839	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	613	867	1377	-	-	-
Mov Cap-2 Maneuver	613	-	-	-	-	-
Stage 1	832	-	-	-	-	-
Stage 2	839	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	10.3	1.7		0		
HCM LOS	B					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	1377	-	613	867	-	-
HCM Lane V/C Ratio	0.025	-	0.039	0.024	-	-
HCM Control Delay (s)	7.7	0	11.1	9.3	-	-
HCM Lane LOS	A	A	B	A	-	-
HCM 95th %tile Q(veh)	0.1	-	0.1	0.1	-	-

6: Plaza Dr & Site Access A

2026 TOTAL (Background plus Site Full Build-out) - PM Peak Hour

HCM 6th TWSC

Intersection						
Int Delay, s/veh	2.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	22	98	0	38	42
Future Vol, veh/h	0	22	98	0	38	42
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	24	107	0	41	46
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	235	107	0	0	107	0
Stage 1	107	-	-	-	-	-
Stage 2	128	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	753	947	-	-	1484	-
Stage 1	917	-	-	-	-	-
Stage 2	898	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	732	947	-	-	1484	-
Mov Cap-2 Maneuver	732	-	-	-	-	-
Stage 1	917	-	-	-	-	-
Stage 2	873	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	8.9	0		3.6		
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	947	1484	-	
HCM Lane V/C Ratio	-	-	0.025	0.028	-	
HCM Control Delay (s)	-	-	8.9	7.5	0	
HCM Lane LOS	-	-	A	A	A	
HCM 95th %tile Q(veh)	-	-	0.1	0.1	-	

APPENDIX G: Future 2031 Level-of-Service Analysis

1: Site Access A/Plaza Dr & Superstition Blvd
2031 BACKGROUND ONLY - AM Peak Hour

HCM 6th TWSC

Intersection												
Int Delay, s/veh	2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	20	320	91	42	345	7	27	4	20	11	7	13
Future Vol, veh/h	20	320	91	42	345	7	27	4	20	11	7	13
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	0	-	-	0	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	22	348	99	46	375	8	29	4	22	12	8	14

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	383	0	0	447	0	0	726	917	224	691	962	192
Stage 1	-	-	-	-	-	-	442	442	-	471	471	-
Stage 2	-	-	-	-	-	-	284	475	-	220	491	-
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32
Pot Cap-1 Maneuver	1172	-	-	1110	-	-	312	270	779	331	254	817
Stage 1	-	-	-	-	-	-	564	575	-	542	558	-
Stage 2	-	-	-	-	-	-	699	556	-	762	546	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1172	-	-	1110	-	-	285	254	779	303	239	817
Mov Cap-2 Maneuver	-	-	-	-	-	-	285	254	-	303	239	-
Stage 1	-	-	-	-	-	-	553	564	-	532	535	-
Stage 2	-	-	-	-	-	-	649	533	-	721	536	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.4			0.9			15.5			14.9		
HCM LOS							C			B		

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)	285	579	1172	-	-	1110	-	-	303	442
HCM Lane V/C Ratio	0.103	0.045	0.019	-	-	0.041	-	-	0.039	0.049
HCM Control Delay (s)	19.1	11.5	8.1	-	-	8.4	-	-	17.4	13.6
HCM Lane LOS	C	B	A	-	-	A	-	-	C	B
HCM 95th %tile Q(veh)	0.3	0.1	0.1	-	-	0.1	-	-	0.1	0.2

2: Plaza Dr & Existing Driveway
2031 BACKGROUND ONLY - AM Peak Hour

HCM 6th TWSC

Intersection						
Int Delay, s/veh	1.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	15	23	10	34	139	6
Future Vol, veh/h	15	23	10	34	139	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	16	25	11	37	151	7

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	214	155	158	0	-	0
Stage 1	155	-	-	-	-	-
Stage 2	59	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	774	891	1422	-	-	-
Stage 1	873	-	-	-	-	-
Stage 2	964	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	768	891	1422	-	-	-
Mov Cap-2 Maneuver	768	-	-	-	-	-
Stage 1	866	-	-	-	-	-
Stage 2	964	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	9.5	1.7	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1422	-	838	-	-
HCM Lane V/C Ratio	0.008	-	0.049	-	-
HCM Control Delay (s)	7.6	0	9.5	-	-
HCM Lane LOS	A	A	A	-	-
HCM 95th %tile Q(veh)	0	-	0.2	-	-

3: Idaho Rd & Apache Tr

2031 BACKGROUND ONLY - AM Peak Hour

HCM 6th Signalized Intersection Summary

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	59	42	11	87	55	4	17	369	127	4	402	77
Future Volume (veh/h)	59	42	11	87	55	4	17	369	127	4	402	77
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	64	46	12	95	60	4	18	401	138	4	437	84
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	505	488	122	507	587	39	490	1267	537	517	1009	192
Arrive On Green	0.17	0.17	0.17	0.17	0.17	0.17	0.34	0.34	0.34	0.34	0.34	0.34
Sat Flow, veh/h	1338	2814	706	1345	3384	223	881	3741	1585	866	2977	568
Grp Volume(v), veh/h	64	28	30	95	31	33	18	401	138	4	260	261
Grp Sat Flow(s),veh/h/ln	1338	1777	1743	1345	1777	1830	881	1870	1585	866	1777	1768
Q Serve(g_s), s	1.0	0.3	0.4	1.6	0.4	0.4	0.4	2.0	1.6	0.1	2.8	2.8
Cycle Q Clear(g_c), s	1.4	0.3	0.4	1.9	0.4	0.4	3.2	2.0	1.6	2.0	2.8	2.8
Prop In Lane	1.00		0.41	1.00		0.12	1.00		1.00	1.00		0.32
Lane Grp Cap(c), veh/h	505	308	302	507	308	317	490	1267	537	517	602	599
V/C Ratio(X)	0.13	0.09	0.10	0.19	0.10	0.10	0.04	0.32	0.26	0.01	0.43	0.44
Avail Cap(c_a), veh/h	2176	2528	2480	2187	2528	2604	1731	6538	2770	1738	3105	3090
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	9.2	8.5	8.5	9.4	8.6	8.6	7.6	6.0	5.9	6.8	6.3	6.3
Incr Delay (d2), s/veh	0.1	0.1	0.1	0.2	0.1	0.1	0.0	0.1	0.3	0.0	0.5	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	0.1	0.1	0.3	0.1	0.1	0.0	0.2	0.2	0.0	0.4	0.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	9.3	8.7	8.7	9.5	8.7	8.7	7.6	6.2	6.1	6.8	6.8	6.8
LnGrp LOS	A	A	A	A	A	A	A	A	A	A	A	A
Approach Vol, veh/h	122			159			557			525		
Approach Delay, s/veh	9.0			9.2			6.2			6.8		
Approach LOS	A			A			A			A		
Timer - Assigned Phs	2			4			6			8		
Phs Duration (G+Y+Rc), s	14.3			10.3			14.3			10.3		
Change Period (Y+Rc), s	6.0			6.0			6.0			6.0		
Max Green Setting (Gmax), s	43.0			35.0			43.0			35.0		
Max Q Clear Time (g_c+I1), s	5.2			3.4			4.8			3.9		
Green Ext Time (p_c), s	3.1			0.5			3.1			0.6		
Intersection Summary												
HCM 6th Ctrl Delay	7.0											
HCM 6th LOS	A											

4: Apache Tr/Phelps Dr & Old West Hwy
2031 BACKGROUND ONLY - AM Peak Hour

HCM 6th Signalized Intersection Summary



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↰ ↱ ↲ ↳			↰ ↱ ↲ ↳			↰ ↱ ↳			↰ ↳		↱ ↲
Traffic Volume (veh/h)	77	511	58	27	595	39	72	63	32	39	38	97
Future Volume (veh/h)	77	511	58	27	595	39	72	63	32	39	38	97
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	84	555	63	29	647	42	60	93	35	42	89	73
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	173	1031	116	52	982	63	593	1028	368	612	718	608
Arrive On Green	0.05	0.22	0.22	0.03	0.20	0.20	0.04	0.39	0.39	0.04	0.38	0.38
Sat Flow, veh/h	3456	4658	522	1781	4901	316	1781	2630	942	1781	1870	1585
Grp Volume(v), veh/h	84	404	214	29	448	241	60	65	63	42	89	73
Grp Sat Flow(s),veh/h/ln	1728	1702	1776	1781	1702	1813	1781	1870	1701	1781	1870	1585
Q Serve(g_s), s	2.1	9.1	9.3	1.4	10.5	10.6	1.7	1.9	2.0	1.2	2.7	2.6
Cycle Q Clear(g_c), s	2.1	9.1	9.3	1.4	10.5	10.6	1.7	1.9	2.0	1.2	2.7	2.6
Prop In Lane	1.00		0.29	1.00		0.17	1.00		0.55	1.00		1.00
Lane Grp Cap(c), veh/h	173	753	393	52	682	363	593	731	665	612	718	608
V/C Ratio(X)	0.49	0.54	0.55	0.56	0.66	0.66	0.10	0.09	0.10	0.07	0.12	0.12
Avail Cap(c_a), veh/h	437	1488	776	225	1488	793	699	731	665	731	731	620
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	40.2	29.9	30.0	41.7	32.0	32.1	14.8	16.7	16.7	15.0	17.3	17.3
Incr Delay (d2), s/veh	2.1	0.6	1.2	9.2	1.1	2.1	0.1	0.2	0.3	0.0	0.1	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.9	3.7	4.0	0.7	4.3	4.8	0.7	0.8	0.8	0.5	1.1	0.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	42.3	30.5	31.2	50.9	33.1	34.1	14.8	16.9	17.0	15.0	17.4	17.4
LnGrp LOS	D	C	C	D	C	C	B	B	B	B	B	B
Approach Vol, veh/h	702		718			188			204			
Approach Delay, s/veh	32.1		34.2			16.3			16.9			
Approach LOS	C		C			B			B			
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	40.2	41.0	9.5	26.2	10.8	40.4	11.3	24.4				
Change Period (Y+Rc), s	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0				
Max Green Setting (Gmax), s	34.0	34.0	11.0	38.0	9.0	34.0	11.0	38.0				
Max Q Clear Time (g_c+I), s	13.2	4.0	3.4	11.3	3.7	4.7	4.1	12.6				
Green Ext Time (p_c), s	0.0	0.7	0.0	4.3	0.0	0.7	0.1	4.8				

Intersection Summary

HCM 6th Ctrl Delay 29.6

HCM 6th LOS C

Notes

User approved volume balancing among the lanes for turning movement.

1: Site Access A/Plaza Dr & Superstition Blvd
2031 BACKGROUND ONLY - PM Peak Hour

HCM 6th TWSC

Intersection												
Int Delay, s/veh	3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	17	475	28	20	348	23	59	15	49	13	6	25
Future Vol, veh/h	17	475	28	20	348	23	59	15	49	13	6	25
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	0	-	-	0	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	18	516	30	22	378	25	64	16	53	14	7	27

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	403	0	0	546	0	0	804	1014	273	737	1017	202
Stage 1	-	-	-	-	-	-	567	567	-	435	435	-
Stage 2	-	-	-	-	-	-	237	447	-	302	582	-
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32
Pot Cap-1 Maneuver	1152	-	-	1019	-	-	274	237	725	307	236	805
Stage 1	-	-	-	-	-	-	476	505	-	570	579	-
Stage 2	-	-	-	-	-	-	745	572	-	682	497	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1152	-	-	1019	-	-	252	228	725	262	227	805
Mov Cap-2 Maneuver	-	-	-	-	-	-	252	228	-	262	227	-
Stage 1	-	-	-	-	-	-	468	497	-	561	566	-
Stage 2	-	-	-	-	-	-	696	559	-	602	489	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0.4			18.7			14.3		
HCM LOS							C			B		

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)	252	480	1152	-	-	1019	-	-	262	539
HCM Lane V/C Ratio	0.254	0.145	0.016	-	-	0.021	-	-	0.054	0.063
HCM Control Delay (s)	24.1	13.8	8.2	-	-	8.6	-	-	19.5	12.1
HCM Lane LOS	C	B	A	-	-	A	-	-	C	B
HCM 95th %tile Q(veh)	1	0.5	0	-	-	0.1	-	-	0.2	0.2

2: Plaza Dr & Existing Driveway
2031 BACKGROUND ONLY - PM Peak Hour

HCM 6th TWSC

Intersection						
Int Delay, s/veh	1.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	15	21	7	110	39	13
Future Vol, veh/h	15	21	7	110	39	13
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	16	23	8	120	42	14

Major/Minor	Minor2	Major1	Major2
Conflicting Flow All	185	49	56
Stage 1	49	-	-
Stage 2	136	-	-
Critical Hdwy	6.42	6.22	4.12
Critical Hdwy Stg 1	5.42	-	-
Critical Hdwy Stg 2	5.42	-	-
Follow-up Hdwy	3.518	3.318	2.218
Pot Cap-1 Maneuver	804	1020	1549
Stage 1	973	-	-
Stage 2	890	-	-
Platoon blocked, %			
Mov Cap-1 Maneuver	799	1020	1549
Mov Cap-2 Maneuver	799	-	-
Stage 1	967	-	-
Stage 2	890	-	-

Approach	EB	NB	SB
HCM Control Delay, s	9.1	0.4	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1549	-	915	-	-
HCM Lane V/C Ratio	0.005	-	0.043	-	-
HCM Control Delay (s)	7.3	0	9.1	-	-
HCM Lane LOS	A	A	A	-	-
HCM 95th %tile Q(veh)	0	-	0.1	-	-

3: Idaho Rd & Apache Tr

2031 BACKGROUND ONLY - PM Peak Hour

HCM 6th Signalized Intersection Summary

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	69	61	15	118	72	0	20	428	103	11	498	91
Future Volume (veh/h)	69	61	15	118	72	0	20	428	103	11	498	91
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	75	66	16	128	78	0	22	465	112	12	541	99
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	263	430	101	261	534	0	590	2617	1109	699	2100	383
Arrive On Green	0.15	0.15	0.15	0.15	0.15	0.00	0.70	0.70	0.70	0.70	0.70	0.70
Sat Flow, veh/h	1321	2857	669	1316	3647	0	789	3741	1585	928	3001	547
Grp Volume(v), veh/h	75	40	42	128	78	0	22	465	112	12	319	321
Grp Sat Flow(s),veh/h/ln	1321	1777	1750	1316	1777	0	789	1870	1585	928	1777	1772
Q Serve(g_s), s	4.2	1.6	1.7	7.5	1.5	0.0	0.8	3.4	1.8	0.4	5.3	5.3
Cycle Q Clear(g_c), s	5.7	1.6	1.7	9.2	1.5	0.0	6.2	3.4	1.8	3.8	5.3	5.3
Prop In Lane	1.00		0.38	1.00		0.00	1.00		1.00	1.00		0.31
Lane Grp Cap(c), veh/h	263	267	263	261	534	0	590	2617	1109	699	1243	1240
V/C Ratio(X)	0.28	0.15	0.16	0.49	0.15	0.00	0.04	0.18	0.10	0.02	0.26	0.26
Avail Cap(c_a), veh/h	758	932	918	753	1865	0	590	2617	1109	699	1243	1240
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	32.0	29.6	29.6	33.6	29.5	0.0	5.5	4.1	3.9	4.8	4.4	4.4
Incr Delay (d2), s/veh	0.6	0.3	0.3	1.4	0.1	0.0	0.1	0.1	0.2	0.0	0.5	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.3	0.7	0.7	2.4	0.6	0.0	0.1	1.1	0.5	0.1	1.7	1.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	32.6	29.8	29.9	35.0	29.7	0.0	5.7	4.3	4.1	4.8	4.9	4.9
LnGrp LOS	C	C	C	D	C	A	A	A	A	A	A	A
Approach Vol, veh/h	157			206			599			652		
Approach Delay, s/veh	31.2			33.0			4.3			4.9		
Approach LOS	C			C			A			A		
Timer - Assigned Phs	2			4			6			8		
Phs Duration (G+Y+Rc), s	62.0			18.0			62.0			18.0		
Change Period (Y+Rc), s	6.0			6.0			6.0			6.0		
Max Green Setting (Gmax), s	56.0			42.0			56.0			42.0		
Max Q Clear Time (g_c+I1), s	8.2			7.7			7.3			11.2		
Green Ext Time (p_c), s	4.1			0.7			4.7			0.9		
Intersection Summary												
HCM 6th Ctrl Delay	10.8											
HCM 6th LOS	B											
Notes												

4: Apache Tr/Phelps Dr & Old West Hwy
2031 BACKGROUND ONLY - PM Peak Hour

HCM 6th Signalized Intersection Summary



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↰ ↱ ↲ ↳			↰ ↱ ↲ ↳			↰ ↱ ↳		↰ ↱ ↳		↰ ↱ ↳	
Traffic Volume (veh/h)	76	809	113	87	560	55	179	80	91	66	77	104
Future Volume (veh/h)	76	809	113	87	560	55	179	80	91	66	77	104
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	83	879	123	95	609	60	127	182	99	72	106	98
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	161	1203	168	123	1362	133	533	759	393	448	569	482
Arrive On Green	0.05	0.27	0.27	0.07	0.29	0.29	0.07	0.33	0.33	0.04	0.30	0.30
Sat Flow, veh/h	3456	4530	631	1781	4730	462	1781	2321	1203	1781	1870	1585
Grp Volume(v), veh/h	83	660	342	95	437	232	127	145	136	72	106	98
Grp Sat Flow(s),veh/h/ln	1728	1702	1757	1781	1702	1787	1781	1870	1654	1781	1870	1585
Q Serve(g_s), s	2.2	16.8	16.9	5.0	10.0	10.1	4.6	5.4	5.8	2.6	4.0	4.4
Cycle Q Clear(g_c), s	2.2	16.8	16.9	5.0	10.0	10.1	4.6	5.4	5.8	2.6	4.0	4.4
Prop In Lane	1.00		0.36	1.00		0.26	1.00		0.73	1.00		1.00
Lane Grp Cap(c), veh/h	161	904	467	123	980	515	533	611	541	448	569	482
V/C Ratio(X)	0.51	0.73	0.73	0.77	0.45	0.45	0.24	0.24	0.25	0.16	0.19	0.20
Avail Cap(c_a), veh/h	689	1286	664	355	1286	675	563	611	541	518	569	482
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	44.4	31.9	31.9	43.6	27.7	27.8	20.4	23.4	23.5	21.1	24.4	24.6
Incr Delay (d2), s/veh	2.5	1.3	2.5	9.8	0.3	0.6	0.2	0.9	1.1	0.2	0.7	0.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.0	6.9	7.4	2.5	4.0	4.3	1.9	2.5	2.4	1.1	1.9	1.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	46.9	33.1	34.4	53.5	28.0	28.4	20.6	24.3	24.6	21.3	25.2	25.5
LnGrp LOS	D	C	C	D	C	C	C	C	C	C	C	C
Approach Vol, veh/h	1085			764			408			276		
Approach Delay, s/veh	34.6			31.3			23.3			24.3		
Approach LOS	C			C			C			C		
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	11.3	38.1	13.6	32.3	13.4	36.0	11.4	34.4				
Change Period (Y+Rc), s	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0				
Max Green Setting (Gmax), s	29.0	29.0	19.0	36.0	8.0	29.0	19.0	36.0				
Max Q Clear Time (g_c+14.6)	14.6	7.8	7.0	18.9	6.6	6.4	4.2	12.1				
Green Ext Time (p_c), s	0.0	1.6	0.2	6.4	0.0	0.8	0.2	4.5				

Intersection Summary

HCM 6th Ctrl Delay	30.6
HCM 6th LOS	C

Notes

User approved volume balancing among the lanes for turning movement.

1: Site Access A/Plaza Dr & Superstition Blvd
2031 TOTAL (Background plus Site Full Build-out) - AM Peak Hour

HCM 6th TWSC

Intersection												
Int Delay, s/veh	2.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	20	320	100	43	345	7	58	6	23	11	8	13
Future Vol, veh/h	20	320	100	43	345	7	58	6	23	11	8	13
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	0	-	-	0	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	22	348	109	47	375	8	63	7	25	12	9	14
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	383	0	0	457	0	0	733	924	229	695	974	192
Stage 1	-	-	-	-	-	-	447	447	-	473	473	-
Stage 2	-	-	-	-	-	-	286	477	-	222	501	-
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32
Pot Cap-1 Maneuver	1172	-	-	1100	-	-	309	268	774	329	250	817
Stage 1	-	-	-	-	-	-	560	572	-	541	557	-
Stage 2	-	-	-	-	-	-	697	554	-	760	541	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1172	-	-	1100	-	-	281	252	774	298	235	817
Mov Cap-2 Maneuver	-	-	-	-	-	-	281	252	-	298	235	-
Stage 1	-	-	-	-	-	-	549	561	-	531	533	-
Stage 2	-	-	-	-	-	-	645	530	-	713	531	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.4			0.9			18.4			15.3		
HCM LOS							C			C		
Minor Lane/Major Mvmt	NBLn1		NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2	
Capacity (veh/h)	281		542	1172	-	-	1100	-	-	298	420	
HCM Lane V/C Ratio	0.224		0.058	0.019	-	-	0.042	-	-	0.04	0.054	
HCM Control Delay (s)	21.5		12.1	8.1	-	-	8.4	-	-	17.6	14.1	
HCM Lane LOS	C		B	A	-	-	A	-	-	C	B	
HCM 95th %tile Q(veh)	0.8		0.2	0.1	-	-	0.1	-	-	0.1	0.2	

2: Plaza Dr & Existing Driveway

2031 TOTAL (Background plus Site Full Build-out) - AM Peak Hour

HCM 6th TWSC

Intersection						
Int Delay, s/veh	1.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	15	23	10	34	139	6
Future Vol, veh/h	15	23	10	34	139	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	16	25	11	37	151	7
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	214	155	158	0	-	0
Stage 1	155	-	-	-	-	-
Stage 2	59	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	774	891	1422	-	-	-
Stage 1	873	-	-	-	-	-
Stage 2	964	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	768	891	1422	-	-	-
Mov Cap-2 Maneuver	768	-	-	-	-	-
Stage 1	866	-	-	-	-	-
Stage 2	964	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	9.5	1.7		0		
HCM LOS	A					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	1422	-	838	-	-	
HCM Lane V/C Ratio	0.008	-	0.049	-	-	
HCM Control Delay (s)	7.6	0	9.5	-	-	
HCM Lane LOS	A	A	A	-	-	
HCM 95th %tile Q(veh)	0	-	0.2	-	-	

3: Idaho Rd & Apache Tr

2031 TOTAL (Background plus Site Full Build-out) - AM Peak Hour

Queues

									
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	72	89	95	67	25	415	124	4	523
v/c Ratio	0.21	0.10	0.29	0.07	0.06	0.26	0.17	0.01	0.32
Control Delay	11.0	6.6	11.9	8.7	7.7	7.6	2.8	7.0	7.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	11.0	6.6	11.9	8.7	7.7	7.6	2.8	7.0	7.5
Queue Length 50th (ft)	8	3	11	3	2	24	0	1	28
Queue Length 95th (ft)	29	13	36	13	12	51	19	4	58
Internal Link Dist (ft)		73		91		2048			1141
Turn Bay Length (ft)									
Base Capacity (vph)	1316	3310	1288	3486	853	3373	1441	931	3451
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.05	0.03	0.07	0.02	0.03	0.12	0.09	0.00	0.15
Intersection Summary									

3: Idaho Rd & Apache Tr

2031 TOTAL (Background plus Site Full Build-out) - AM Peak Hour HCM 6th Signalized Intersection Summary

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	66	50	32	87	58	4	23	369	127	4	402	79
Future Volume (veh/h)	66	50	32	87	58	4	23	369	127	4	402	79
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	72	54	35	95	63	4	25	401	138	4	437	86
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	504	383	227	490	606	38	488	1283	544	516	1017	199
Arrive On Green	0.18	0.18	0.18	0.18	0.18	0.18	0.34	0.34	0.34	0.34	0.34	0.34
Sat Flow, veh/h	1334	2145	1273	1308	3395	214	879	3741	1585	866	2964	579
Grp Volume(v), veh/h	72	44	45	95	33	34	25	401	138	4	261	262
Grp Sat Flow(s),veh/h/ln	1334	1777	1641	1308	1777	1832	879	1870	1585	866	1777	1766
Q Serve(g_s), s	1.2	0.5	0.6	1.7	0.4	0.4	0.6	2.0	1.6	0.1	2.8	2.9
Cycle Q Clear(g_c), s	1.6	0.5	0.6	2.2	0.4	0.4	3.4	2.0	1.6	2.1	2.8	2.9
Prop In Lane	1.00		0.78	1.00		0.12	1.00		1.00	1.00		0.33
Lane Grp Cap(c), veh/h	504	317	293	490	317	327	488	1283	544	516	609	606
V/C Ratio(X)	0.14	0.14	0.15	0.19	0.10	0.10	0.05	0.31	0.25	0.01	0.43	0.43
Avail Cap(c_a), veh/h	2129	2480	2291	2083	2480	2557	1694	6416	2719	1705	3047	3029
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	9.3	8.7	8.7	9.6	8.6	8.6	7.7	6.1	5.9	6.8	6.3	6.4
Incr Delay (d2), s/veh	0.1	0.2	0.2	0.2	0.1	0.1	0.0	0.1	0.2	0.0	0.5	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.2	0.1	0.1	0.3	0.1	0.1	0.1	0.3	0.2	0.0	0.4	0.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	9.4	8.9	8.9	9.8	8.8	8.8	7.7	6.2	6.2	6.8	6.8	6.8
LnGrp LOS	A	A	A	A	A	A	A	A	A	A	A	A
Approach Vol, veh/h	161			162			564			527		
Approach Delay, s/veh	9.1			9.4			6.3			6.8		
Approach LOS	A			A			A			A		
Timer - Assigned Phs	2			4			6			8		
Phs Duration (G+Y+Rc), s	14.6			10.5			14.6			10.5		
Change Period (Y+Rc), s	6.0			6.0			6.0			6.0		
Max Green Setting (Gmax), s	43.0			35.0			43.0			35.0		
Max Q Clear Time (g_c+I1), s	5.4			3.6			4.9			4.2		
Green Ext Time (p_c), s	3.2			0.7			3.1			0.6		
Intersection Summary												
HCM 6th Ctrl Delay				7.2								
HCM 6th LOS				A								
Notes												

4: Apache Tr/Phelps Dr & Old West Hwy

2031 TOTAL (Background plus Site Full Build-out) - AM Peak Hour

Queues

									
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	90	618	29	692	61	123	49	90	84
v/c Ratio	0.30	0.49	0.21	0.63	0.11	0.09	0.08	0.14	0.12
Control Delay	46.1	31.3	47.8	36.3	15.1	10.8	14.9	16.3	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	46.1	31.3	47.8	36.3	15.1	10.8	14.9	16.3	0.4
Queue Length 50th (ft)	27	121	17	142	20	14	15	22	0
Queue Length 95th (ft)	55	167	48	192	51	35	39	66	0
Internal Link Dist (ft)		15		1238		264		130	
Turn Bay Length (ft)									
Base Capacity (vph)	428	2167	220	2173	579	1376	621	656	680
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.21	0.29	0.13	0.32	0.11	0.09	0.08	0.14	0.12
Intersection Summary									

4: Apache Tr/Phelps Dr & Old West Hwy

2031 TOTAL (Background plus Site Full Build-out) - AM Peak Hour HCM 6th Signalized Intersection Summary

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	83	511	58	27	595	41	72	65	32	45	43	117
Future Volume (veh/h)	83	511	58	27	595	41	72	65	32	45	43	117
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	90	555	63	29	647	45	61	94	35	49	107	87
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	175	1037	116	51	979	68	573	1025	363	613	719	609
Arrive On Green	0.05	0.22	0.22	0.03	0.20	0.20	0.04	0.39	0.39	0.04	0.38	0.38
Sat Flow, veh/h	3456	4658	522	1781	4877	337	1781	2638	935	1781	1870	1585
Grp Volume(v), veh/h	90	404	214	29	450	242	61	65	64	49	107	87
Grp Sat Flow(s),veh/h/ln	1728	1702	1776	1781	1702	1810	1781	1870	1702	1781	1870	1585
Q Serve(g_s), s	2.2	9.2	9.3	1.4	10.7	10.8	1.8	1.9	2.1	1.4	3.3	3.1
Cycle Q Clear(g_c), s	2.2	9.2	9.3	1.4	10.7	10.8	1.8	1.9	2.1	1.4	3.3	3.1
Prop In Lane	1.00		0.29	1.00		0.19	1.00		0.55	1.00		1.00
Lane Grp Cap(c), veh/h	175	758	395	51	683	363	573	727	662	613	719	609
V/C Ratio(X)	0.51	0.53	0.54	0.56	0.66	0.67	0.11	0.09	0.10	0.08	0.15	0.14
Avail Cap(c_a), veh/h	435	1479	772	224	1479	786	678	727	662	726	727	616
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	40.5	30.0	30.1	41.9	32.2	32.2	14.9	16.9	17.0	14.9	17.6	17.5
Incr Delay (d2), s/veh	2.3	0.6	1.2	9.3	1.1	2.1	0.1	0.2	0.3	0.1	0.1	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.0	3.7	4.0	0.7	4.4	4.8	0.7	0.9	0.8	0.6	1.4	1.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	42.8	30.6	31.2	51.2	33.3	34.3	14.9	17.2	17.3	15.0	17.7	17.7
LnGrp LOS	D	C	C	D	C	C	B	B	B	B	B	B
Approach Vol, veh/h	708			721			190			243		
Approach Delay, s/veh	32.3			34.4			16.5			17.1		
Approach LOS	C			C			B			B		
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.5	41.0	9.5	26.5	10.9	40.6	11.4	24.6				
Change Period (Y+Rc), s	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0				
Max Green Setting (Gmax), s	9.0	34.0	11.0	38.0	9.0	34.0	11.0	38.0				
Max Q Clear Time (g_c+I1), s	3.4	4.1	3.4	11.3	3.8	5.3	4.2	12.8				
Green Ext Time (p_c), s	0.0	0.7	0.0	4.3	0.0	0.9	0.1	4.8				

Intersection Summary

HCM 6th Ctrl Delay 29.5

HCM 6th LOS C

Notes

User approved volume balancing among the lanes for turning movement.

5: Apache Tr & Site Access B

2031 TOTAL (Background plus Site Full Build-out) - AM Peak Hour

HCM 6th TWSC

Intersection						
Int Delay, s/veh	2.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	36	31	10	113	151	11
Future Vol, veh/h	36	31	10	113	151	11
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	39	34	11	123	164	12
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	315	170	176	0	-	0
Stage 1	170	-	-	-	-	-
Stage 2	145	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	678	874	1400	-	-	-
Stage 1	860	-	-	-	-	-
Stage 2	882	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	673	874	1400	-	-	-
Mov Cap-2 Maneuver	673	-	-	-	-	-
Stage 1	853	-	-	-	-	-
Stage 2	882	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	10.1	0.6		0		
HCM LOS	B					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	1400	-	673	874	-	-
HCM Lane V/C Ratio	0.008	-	0.058	0.039	-	-
HCM Control Delay (s)	7.6	0	10.7	9.3	-	-
HCM Lane LOS	A	A	B	A	-	-
HCM 95th %tile Q(veh)	0	-	0.2	0.1	-	-

6: Plaza Dr & Site Access A

2031 TOTAL (Background plus Site Full Build-out) - AM Peak Hour

HCM 6th TWSC

Intersection						
Int Delay, s/veh	1.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	36	49	0	11	145
Future Vol, veh/h	0	36	49	0	11	145
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	39	53	0	12	158
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	235	53	0	0	53	0
Stage 1	53	-	-	-	-	-
Stage 2	182	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	753	1014	-	-	1553	-
Stage 1	970	-	-	-	-	-
Stage 2	849	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	747	1014	-	-	1553	-
Mov Cap-2 Maneuver	747	-	-	-	-	-
Stage 1	970	-	-	-	-	-
Stage 2	842	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	8.7	0		0.5		
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	1014	1553	-	
HCM Lane V/C Ratio	-	-	0.039	0.008	-	
HCM Control Delay (s)	-	-	8.7	7.3	0	
HCM Lane LOS	-	-	A	A	A	
HCM 95th %tile Q(veh)	-	-	0.1	0	-	

1: Site Access A/Plaza Dr & Superstition Blvd
2031 TOTAL (Background plus Site Full Build-out) - PM Peak Hour

HCM 6th TWSC

Intersection												
Int Delay, s/veh	3.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↰	↑↱		↰	↑↱		↰	↱		↰	↱	
Traffic Vol, veh/h	17	475	61	23	348	23	78	16	51	13	8	25
Future Vol, veh/h	17	475	61	23	348	23	78	16	51	13	8	25
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	0	-	-	0	-	-	0	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	18	516	66	25	378	25	85	17	55	14	9	27
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	403	0	0	582	0	0	829	1038	291	744	1059	202
Stage 1	-	-	-	-	-	-	585	585	-	441	441	-
Stage 2	-	-	-	-	-	-	244	453	-	303	618	-
Critical Hdwy	4.14	-	-	4.14	-	-	7.54	6.54	6.94	7.54	6.54	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.54	5.54	-	6.54	5.54	-
Follow-up Hdwy	2.22	-	-	2.22	-	-	3.52	4.02	3.32	3.52	4.02	3.32
Pot Cap-1 Maneuver	1152	-	-	988	-	-	263	229	706	303	223	805
Stage 1	-	-	-	-	-	-	464	496	-	565	575	-
Stage 2	-	-	-	-	-	-	738	568	-	681	479	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1152	-	-	988	-	-	239	220	706	254	214	805
Mov Cap-2 Maneuver	-	-	-	-	-	-	239	220	-	254	214	-
Stage 1	-	-	-	-	-	-	457	488	-	556	561	-
Stage 2	-	-	-	-	-	-	684	554	-	596	471	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0.5			21.7			15.1		
HCM LOS							C			C		
Minor Lane/Major Mvmt	NBLn1 NBLn2		EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2		
Capacity (veh/h)	239 462		1152	-	-	988	-	-	254	482		
HCM Lane V/C Ratio	0.355 0.158		0.016	-	-	0.025	-	-	0.056	0.074		
HCM Control Delay (s)	28.1 14.2		8.2	-	-	8.7	-	-	20	13.1		
HCM Lane LOS	D B		A	-	-	A	-	-	C	B		
HCM 95th %tile Q(veh)	1.5 0.6		0	-	-	0.1	-	-	0.2	0.2		

2: Plaza Dr & Existing Driveway

2031 TOTAL (Background plus Site Full Build-out) - PM Peak Hour

HCM 6th TWSC

Intersection						
Int Delay, s/veh	1.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	15	21	7	110	39	13
Future Vol, veh/h	15	21	7	110	39	13
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	16	23	8	120	42	14

Major/Minor	Minor2	Major1	Major2
Conflicting Flow All	185	49	56
Stage 1	49	-	-
Stage 2	136	-	-
Critical Hdwy	6.42	6.22	4.12
Critical Hdwy Stg 1	5.42	-	-
Critical Hdwy Stg 2	5.42	-	-
Follow-up Hdwy	3.518	3.318	2.218
Pot Cap-1 Maneuver	804	1020	1549
Stage 1	973	-	-
Stage 2	890	-	-
Platoon blocked, %			
Mov Cap-1 Maneuver	799	1020	1549
Mov Cap-2 Maneuver	799	-	-
Stage 1	967	-	-
Stage 2	890	-	-

Approach	EB	NB	SB
HCM Control Delay, s	9.1	0.4	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1549	-	915	-	-
HCM Lane V/C Ratio	0.005	-	0.043	-	-
HCM Control Delay (s)	7.3	0	9.1	-	-
HCM Lane LOS	A	A	A	-	-
HCM 95th %tile Q(veh)	0	-	0.1	-	-

3: Idaho Rd & Apache Tr

2031 TOTAL (Background plus Site Full Build-out) - PM Peak Hour

Queues

									
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT
Lane Group Flow (vph)	79	102	128	88	46	476	101	12	648
v/c Ratio	0.38	0.18	0.62	0.15	0.09	0.20	0.10	0.02	0.27
Control Delay	35.3	21.4	45.1	29.2	5.7	5.2	1.5	5.3	5.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	35.3	21.4	45.1	29.2	5.7	5.2	1.5	5.3	5.3
Queue Length 50th (ft)	36	16	61	20	7	40	0	2	53
Queue Length 95th (ft)	76	37	116	40	22	75	17	8	95
Internal Link Dist (ft)		73		91		2048			1141
Turn Bay Length (ft)									
Base Capacity (vph)	655	1722	647	1786	524	2347	1031	607	2405
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.12	0.06	0.20	0.05	0.09	0.20	0.10	0.02	0.27
Intersection Summary									

3: Idaho Rd & Apache Tr

2031 TOTAL (Background plus Site Full Build-out) - PM Peak Hour HCM 6th Signalized Intersection Summary

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	73	66	28	118	81	0	42	428	103	11	498	98
Future Volume (veh/h)	73	66	28	118	81	0	42	428	103	11	498	98
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	79	72	30	128	88	0	46	465	112	12	541	107
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	268	394	155	260	563	0	578	2593	1099	692	2051	404
Arrive On Green	0.16	0.16	0.16	0.16	0.16	0.00	0.69	0.69	0.69	0.69	0.69	0.69
Sat Flow, veh/h	1309	2489	981	1293	3647	0	783	3741	1585	928	2959	583
Grp Volume(v), veh/h	79	50	52	128	88	0	46	465	112	12	324	324
Grp Sat Flow(s),veh/h/ln	1309	1777	1694	1293	1777	0	783	1870	1585	928	1777	1765
Q Serve(g_s), s	4.5	2.0	2.1	7.7	1.7	0.0	1.9	3.5	1.9	0.4	5.5	5.6
Cycle Q Clear(g_c), s	6.2	2.0	2.1	9.9	1.7	0.0	7.5	3.5	1.9	3.9	5.5	5.6
Prop In Lane	1.00		0.58	1.00		0.00	1.00		1.00	1.00		0.33
Lane Grp Cap(c), veh/h	268	281	268	260	563	0	578	2593	1099	692	1231	1224
V/C Ratio(X)	0.29	0.18	0.19	0.49	0.16	0.00	0.08	0.18	0.10	0.02	0.26	0.26
Avail Cap(c_a), veh/h	742	924	880	727	1847	0	578	2593	1099	692	1231	1224
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	32.0	29.4	29.5	33.8	29.3	0.0	6.1	4.3	4.1	5.0	4.7	4.7
Incr Delay (d2), s/veh	0.6	0.3	0.3	1.4	0.1	0.0	0.3	0.2	0.2	0.0	0.5	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.4	0.8	0.9	2.5	0.7	0.0	0.3	1.1	0.5	0.1	1.8	1.8
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	32.6	29.7	29.9	35.2	29.5	0.0	6.3	4.5	4.3	5.1	5.2	5.2
LnGrp LOS	C	C	C	D	C	A	A	A	A	A	A	A
Approach Vol, veh/h	181				216				623			
Approach Delay, s/veh	31.0				32.9				4.6			
Approach LOS	C				C				A			
Timer - Assigned Phs	2			4			6			8		
Phs Duration (G+Y+Rc), s	62.0			18.8			62.0			18.8		
Change Period (Y+Rc), s	6.0			6.0			6.0			6.0		
Max Green Setting (Gmax), s	56.0			42.0			56.0			42.0		
Max Q Clear Time (g_c+I1), s	9.5			8.2			7.6			11.9		
Green Ext Time (p_c), s	4.3			0.8			4.7			0.9		
Intersection Summary												
HCM 6th Ctrl Delay	11.3											
HCM 6th LOS	B											
Notes												

4: Apache Tr/Phelps Dr & Old West Hwy

2031 TOTAL (Background plus Site Full Build-out) - PM Peak Hour

Queues

									
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	107	1002	95	674	129	257	75	114	100
v/c Ratio	0.37	0.67	0.51	0.47	0.29	0.23	0.17	0.23	0.18
Control Delay	51.0	34.8	55.5	30.5	24.1	13.6	22.5	30.1	1.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	51.0	34.8	55.5	30.5	24.1	13.6	22.5	30.1	1.2
Queue Length 50th (ft)	36	218	62	130	59	35	30	55	0
Queue Length 95th (ft)	68	282	121	172	123	73	70	118	5
Internal Link Dist (ft)		15		1238		264		130	
Turn Bay Length (ft)									
Base Capacity (vph)	634	1763	327	1811	449	1111	445	489	542
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.17	0.57	0.29	0.37	0.29	0.23	0.17	0.23	0.18
Intersection Summary									

4: Apache Tr/Phelps Dr & Old West Hwy

2031 TOTAL (Background plus Site Full Build-out) - PM Peak Hour HCM 6th Signalized Intersection Summary

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	 	  			  			 				
Traffic Volume (veh/h)	98	809	113	87	560	60	179	85	91	69	80	117
Future Volume (veh/h)	98	809	113	87	560	60	179	85	91	69	80	117
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	107	879	123	95	609	65	129	185	99	75	117	107
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	175	1203	168	123	1331	141	525	763	389	447	569	482
Arrive On Green	0.05	0.27	0.27	0.07	0.28	0.28	0.07	0.33	0.33	0.05	0.30	0.30
Sat Flow, veh/h	3456	4530	631	1781	4690	496	1781	2334	1192	1781	1870	1585
Grp Volume(v), veh/h	107	660	342	95	440	234	129	146	138	75	117	107
Grp Sat Flow(s),veh/h/ln	1728	1702	1757	1781	1702	1781	1781	1870	1656	1781	1870	1585
Q Serve(g_s), s	2.9	16.8	17.0	5.0	10.2	10.3	4.7	5.5	5.8	2.7	4.4	4.8
Cycle Q Clear(g_c), s	2.9	16.8	17.0	5.0	10.2	10.3	4.7	5.5	5.8	2.7	4.4	4.8
Prop In Lane	1.00		0.36	1.00		0.28	1.00		0.72	1.00		1.00
Lane Grp Cap(c), veh/h	175	904	466	123	966	506	525	611	541	447	569	482
V/C Ratio(X)	0.61	0.73	0.73	0.77	0.46	0.46	0.25	0.24	0.25	0.17	0.21	0.22
Avail Cap(c_a), veh/h	688	1284	663	355	1284	672	553	611	541	516	569	482
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	44.4	31.9	32.0	43.7	28.1	28.2	20.4	23.5	23.6	21.2	24.7	24.8
Incr Delay (d2), s/veh	3.4	1.3	2.5	9.9	0.3	0.7	0.2	0.9	1.1	0.2	0.8	1.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.3	6.9	7.4	2.5	4.1	4.4	1.9	2.5	2.4	1.1	2.1	1.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	47.8	33.2	34.5	53.5	28.4	28.8	20.6	24.4	24.7	21.3	25.5	25.8
LnGrp LOS	D	C	C	D	C	C	C	C	C	C	C	C
Approach Vol, veh/h	1109			769			413			299		
Approach Delay, s/veh	35.0			31.7			23.3			24.6		
Approach LOS	C			C			C			C		
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	11.3	38.2	13.6	32.3	13.5	36.0	11.8	34.1				
Change Period (Y+Rc), s	7.0	7.0	7.0	7.0	7.0	7.0	7.0	7.0				
Max Green Setting (Gmax), s	8.0	29.0	19.0	36.0	8.0	29.0	19.0	36.0				
Max Q Clear Time (g_c+I1), s	4.7	7.8	7.0	19.0	6.7	6.8	4.9	12.3				
Green Ext Time (p_c), s	0.0	1.6	0.2	6.4	0.0	0.9	0.2	4.6				
Intersection Summary												
HCM 6th Ctrl Delay	30.9											
HCM 6th LOS	C											
Notes												

5: Apache Tr & Site Access B

2031 TOTAL (Background plus Site Full Build-out) - PM Peak Hour

HCM 6th TWSC

Intersection						
Int Delay, s/veh	1.6					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	22	19	32	145	183	38
Future Vol, veh/h	22	19	32	145	183	38
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	24	21	35	158	199	41
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	448	220	240	0	-	0
Stage 1	220	-	-	-	-	-
Stage 2	228	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	568	820	1327	-	-	-
Stage 1	817	-	-	-	-	-
Stage 2	810	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	552	820	1327	-	-	-
Mov Cap-2 Maneuver	552	-	-	-	-	-
Stage 1	793	-	-	-	-	-
Stage 2	810	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	10.7	1.4		0		
HCM LOS	B					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	1327	-	552	820	-	-
HCM Lane V/C Ratio	0.026	-	0.043	0.025	-	-
HCM Control Delay (s)	7.8	0	11.8	9.5	-	-
HCM Lane LOS	A	A	B	A	-	-
HCM 95th %tile Q(veh)	0.1	-	0.1	0.1	-	-

6: Plaza Dr & Site Access A

2031 TOTAL (Background plus Site Full Build-out) - PM Peak Hour

HCM 6th TWSC

Intersection						
Int Delay, s/veh	2.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	22	125	0	38	53
Future Vol, veh/h	0	22	125	0	38	53
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	24	136	0	41	58
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	276	136	0	0	136	0
Stage 1	136	-	-	-	-	-
Stage 2	140	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	714	913	-	-	1448	-
Stage 1	890	-	-	-	-	-
Stage 2	887	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	693	913	-	-	1448	-
Mov Cap-2 Maneuver	693	-	-	-	-	-
Stage 1	890	-	-	-	-	-
Stage 2	861	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	9	0		3.2		
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	913	1448	-	
HCM Lane V/C Ratio	-	-	0.026	0.029	-	
HCM Control Delay (s)	-	-	9	7.6	0	
HCM Lane LOS	-	-	A	A	A	
HCM 95th %tile Q(veh)	-	-	0.1	0.1	-	